

Groundwater Monitoring System Report

Slag Pond Area Big Stone Plant Big Stone City, South Dakota

Prepared for Otter Tail Power Company

December 2016

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Certification

I hereby certify that the monitoring system identified herein has been designed and constructed to meet the requirements of § 257.91, Groundwater monitoring systems, as included in 40 CFR Part 257, Subpart D, Disposal of Coal Combustion Residuals from Electric Utilities.



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Swenson

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Acronyms

Term	Description
Anisotropic	Where there is a directional difference in an aquifer characteristic or parameter; usually
	due to layered geology
ADA	Ash Disposal Area and Surrounding Area
bgs	Below Ground Surface
BMP	Below Measuring Point
CCR	Coal Combustion Residuals
EPA	Environmental Protection Agency
Facility	Big Stone Plant
FGD	Flue Gas Desulfurization
GTU	Glacial Till Unit composed of two subunits consisting of the upper Brown Till and the
	Gray Till below
OTP	Otter Tail Power Company
SCM	Site Conceptual Model
Site	Ponds (Slag Pond, West Brine Pond, East Brine Pond, Reclaim Pond, Cooling Pond,
	Evaporation Pond, and Holding Pond), Landfill (Ash Disposal Area), and Ash Storage
	Area (Temporary Storage Area).
Slag Pond Area	Slag Pond, Temporary Storage Area and Surrounding Area.
TOR	Top of Riser
TSA	Temporary Storage Area

1.0 Introduction

Otter Tail Power Company (OTP) owns and operates Big Stone Plant, a coal-fired generation unit in Big Stone City, South Dakota. The Site location is shown on Figure 1, which includes ponds (Slag Pond, West Brine Pond, East Brine Pond, Reclaim Pond, Cooling Pond, Evaporation Pond, and Holding Pond), landfill (Ash Disposal Area), and ash storage area (Temporary Storage Area).

The Slag Pond is an existing incised CCR surface impoundment and the Ash Disposal Area is an existing CCR landfill at Big Stone Plant that are required to comply with the provisions of the US EPA Coal Combustion Residuals (CCR) Rule (40 CFR Parts 257 and 261 Disposal of Coal Combustion Residuals From Electric Utilities). The Temporary Storage Area (TSA) also is required to comply with the applicable CCR landfill provisions of the CCR Rule since the CCR is not beneficially used offsite, not containerized, and accumulation of CCR occurs on land.

The West Brine Pond, East Brine Pond, Reclaim Pond, Cooling Pond, Evaporation Pond, and Holding Pond are not regulated by the CCR Rule.

The Slag Pond Area consists of the Slag Pond, the TSA, and the area around the Slag Pond and TSA in which the groundwater monitoring system is located. The Slag Pond Area is shown on Figure 2. The Slag Pond Area is a multiunit groundwater monitoring system as allowed by § 257.91 (d). It is not feasible to install a separate groundwater monitoring system for each CCR subunit.

The Ash Disposal Area (ADA) consists of the landfill and the area around the landfill in which the monitoring system is located. The ADA is discussed in a separate report.

This report has been prepared to document hydrogeologic and monitoring system information as required by the CCR Rule for the Slag Pond Area. It describes:

- July, August, and September 2016 field activities
- The site hydrogeology
- The CCR groundwater monitoring system meeting the requirements of the CCR Rule (40 CFR Part 257, US EPA, 2015) at Big Stone Plant (Facility)

1.1 Purpose

This document has been prepared to describe the groundwater monitoring system for the Big Stone Plant Slag Pond Area and how it has been designed to meet the requirements of the CCR Rule (Rule). Specific requirements for groundwater monitoring systems are established in § 257.91, "Groundwater monitoring systems," as follows:

(a) Performance standard. The owner or operator of a CCR unit must install a groundwater monitoring system that consists of a sufficient number of wells, installed at appropriate locations and depths, to yield groundwater samples from the uppermost aquifer that:

- (1) Accurately represent the quality of background groundwater that has not been affected by leakage from a CCR unit. A determination of background quality may include sampling of wells that are not hydraulically upgradient of the CCR management area where:
 - (i) Hydrogeologic conditions do not allow the owner or operator of the CCR unit to determine what wells are hydraulically upgradient; or
 - (ii) Sampling at other wells will provide an indication of background groundwater quality that is as representative or more representative than that provided by the upgradient wells; and
- (2) Accurately represent the quality of groundwater passing the waste boundary of the CCR unit. The downgradient monitoring system must be installed at the waste boundary that ensures detection of groundwater contamination in the uppermost aquifer. All potential contaminant pathways must be monitored.
- (b) The number, spacing, and depths of monitoring systems shall be determined based upon sitespecific technical information that must include thorough characterization of:
 - (1) Aquifer thickness, groundwater flow rate, groundwater flow direction including seasonal and temporal fluctuations in groundwater flow; and
 - (2) Saturated and unsaturated geologic units and fill materials overlying the uppermost aquifer, materials comprising the uppermost aquifer, and materials comprising the confining unit defining the lower boundary of the uppermost aquifer, including, but not limited to, thicknesses, stratigraphy, lithology, hydraulic conductivities, porosities and effective porosities.

1.2 Scope of Work

The scope of work performed for this project includes:

- Collect and review existing information regarding the CCR unit to provide the information required by the CCR Rule.
- Establish and document the groundwater site conceptual model (SCM) that can be used to evaluate site data and design the monitoring system.
- Identify gaps in the existing data and perform additional field tasks to establish a monitoring system as required by the CCR Rule.
- Observe (December 2015) field investigation consisting of the following subtasks:
 - Installation of monitoring wells SLAG 1, SLAG 2, SLAG 3, SLAG 4, and SLAG 5 and soil borings B1, B2, and B3.
 - o Collection of water level elevation data to document groundwater flow directions.

- Observe (July 2016) field investigation consisting of the following subtasks:
 - o Installation of monitoring wells SLAG 2B, SLAG 6, SLAG 7, SLAG 9, and SLAG 9 DEEP. A pilot boring was completed near an existing monitoring well (SLAG 8) to determine the local geology. A downhole video camera was used to investigate the condition of the monitoring well, including the screen, at the existing monitoring well (SLAG 8).
 - Development of monitoring wells proposed to become part of the CCR monitoring system.
 - Collection of geotechnical samples for analysis of parameters such as grain size analysis, vertical hydraulic conductivity, and horizontal hydraulic conductivity.
 - o Collection of water level elevation data to document groundwater flow directions.
 - o Performance of slug tests on select wells to estimate the local hydraulic conductivity.

1.3 Report Contents

Information in this report, assembled in response to the requirements of the CCR Rule, is organized into the following sections:

- Section 1.0 Introduction (this section) which provides an overview.
- Section 2.0 Site Background which provides background information on the Site, including Site operations and setting and geologic and hydrogeological information.
- Section 3.0 Conceptual Model provides a summary of the site conceptual model for the Slag Pond Area.
- Section 4.0 Groundwater Monitoring Well System which provides a description of the CCR monitoring system.
- Section 5.0 References.

2.0 Site Background

2.1 Big Stone Plant CCR Units

The Big Stone Plant burns subbituminous coal to operate its 474 megawatt generating unit. The Big Stone Plant is a zero-discharge facility. The main coal ash products produced are: boiler slag, economizer ash, and a flue gas desulfurization (FGD) product that contains a mixture of fly ash and spent desulfurization material. Boiler slag is conveyed by water, or sluiced, to the Slag Pond (Figure 2). The material is periodically dredged from the pond and stockpiled adjacent to the pond in the TSA. The FGD product is transported by mobile equipment to the on-site CCR landfill (ADA).

2.1.1 Slag Pond Area History and Construction

The Slag Pond was constructed when the Plant began operations in 1975. It was constructed by excavation of the clayey native glacial till. The pond is unlined. Available Slag Pond engineering construction drawings were provided by OTP for review.

The TSA is located adjacent to the northwest edge of the Slag Pond. It is used for dewatering of the slag material prior to loading and selling off-site or hauling to the landfill. It is also unlined and built on native clay till material with a slightly sloped surface graded to allow drainage back into the Slag Pond.

2.2 Site Setting

The Site lies on a glacial drift plain that rises 140 ft. above Big Stone Lake to the east and the Whetstone River to the south. To the west, the ground surface rises 900 ft. in a distance of 15 to 20 miles to the crest of the Coteau des Prairies, a prominent regional highland. The local terrain, prior to construction of the Plant, had changed little since the last glaciers retreated from the area. Natural surface drainage at the Site area is relatively flat with minor changes in elevation.

Information on the Site, geology, and hydrogeology is summarized in the sections below.

2.2.1 Regional Geology

The surficial geology is composed of Late Wisconsin Des Moines Lobe glacial till. Glacial till is unsorted glacial sediment that is derived from the erosion and entrainment of material by the moving ice of a glacier. It is deposited some distance down-ice to form terminal, lateral, medial, and ground moraines. Glacial till is a heterogeneous clay with silt- to boulder-size clasts of glacial origin.

2.2.2 Regional Hydrogeology

Regionally, the Minnesota and Whetstone Rivers are groundwater discharge corridors, and the upland prairies are groundwater recharge areas. Precipitation falling on the uplands seeps to the subsurface and migrates slowly to the discharge areas. Within this regional system, depressions and gullies form local groundwater discharge areas.

Glacial till with relatively low permeability comprises most of the subsurface. Water migrates through till primarily through fractures or through more porous silt and sand seams and lenses. Free water sufficient for even limited domestic use is generally available only from sand seams and lenses within the till.

2.2.3 Site Geology

Figure 3 shows the surface geology at the Site as mapped by the South Dakota Geological Survey (SDGS, 2004). The surface geology underlying most of the Site is mapped as glacial till. Alluvium deposits are mapped to the south and east of the Site. End moraine deposits are shown south of the Slag Pond Area. End moraine deposits consist of glacial till and are characterized by elevated linear ridges with hummocky terrain locally at former ice sheet margins.

Appendix A includes a well completion log for a well that was completed August 24, 1977 near the Plant and Slag Pond. The well log shows the Site to be underlain by approximately 227 ft. of glacial till containing occasional seams and beds of sand. The uppermost till is shown to be a brown till from 0 to 51 ft. below ground surface (bgs), which is underlain by gray till from 51 to 227 ft. bgs. The well log states "black shale with gravel and lignite lenses" from 177 to 227 ft. bgs that is inferred to be glacial till. The well log shows a white coarse sand underlying the till.

Soil borings completed at the Site in December 2015 and July 2016 extended to a maximum depth of 68 ft. bgs. The geology below 68 ft. bgs is inferred from the deeper 1977 well completion log. The shallower glacial till was observed elsewhere at the Site and is described below.

Glacial Till Unit (GTU)

The surficial geology is composed of glacial till. In this report, glacial till is referred to as the Glacial Till Unit (GTU) and consists largely of lean clay with seams and lenses of clayey sand, silty sand, sand, and silt.

The GTU is a continuous lithostratigraphic unit, but it is divided into two separate hydrostratigraphic units for the purpose of this report. The uppermost portion of the GTU is oxidized and more highly fractured than the deeper portion of the till. The uppermost portion of the GTU in this report is called the Brown Till and the lower or deeper unoxidized portion of the GTU is called the Gray Till.

As mentioned in Section 2.2.3, the till is estimated to be 177 ft. thick and could be as thick as 227 ft. at the Site (Appendix A). The thickness of the GTU is anticipated to vary across the Site.

The Brown Till unit is the uppermost material encountered at the Site. The Brown Till unit consists of oxidized yellow to brown lean clay with discontinuous seams and lenses of clayey sand, silty sand, sand, and silt. The Brown Till is clayey with generally low permeability. Although locally higher permeability sand and silt seams and lenses are present within the clay matrix, the seams and lenses are laterally discontinuous and the finer-grained soils in the till likely dictate the effective horizontal hydraulic conductivity at scales that exceed the length of the coarser-grained seams and lenses. The Brown Till is generally more oxidized to brown hues near the water table. The brown oxidized zone transitions with increasing depth (varies by location) to a gray clay till of similar lithology, but it is generally unoxidized.

The Gray Till unit underlies the Brown Till and has a similar lithology as the Brown Till unit, except that is unoxidized. It is also sometimes logged to have a blue hue, which is also indicative of it being unoxidized. The Gray Till may exhibit lower moisture content which may result in lower apparent plasticity. The reduced condition of the Gray Till results because it has little interactions with oxygenated surficial water. The Gray Till would be expected to be less fractured and lower in permeability than the Brown Till as depth increases. Previous studies at the Site (Huntington, 1995) have concluded that the water in the Gray Till near the ADA has a "pre-bomb" tritium signature, indicating that the water in it was recharged prior to 1953. The geology at the ADA is similar to the geology (Brown Till overlying Gray Till) observed at the Slag Pond Area, except thicknesses and occurrences of more permeable seams or lenses are expected to vary.

Below the GTU

As mentioned above, Appendix A includes a well completion log for a well that was completed dated August 24, 1977 near the Plant and Slag Pond. The well log shows a white coarse sand at 227 ft. bgs underlying the Gray Till. It is possible the white coarse sand may be a remnant of Cretaceous Dakota Sandstone or possibly an erroneous classification for weathered granite.

2.3 Slag Pond Area Geology and Hydrogeology

The geology at the Slag Pond Area is comprised of the geology discussed in Section 2.2.3 and the hydrogeology at the Slag Pond Area consists of the more permeable saturated geology.

2.3.1 Geology

The geology at the Slag Pond Area consists of the GTU as described above. Generally, the soil boring logs show that a higher permeability brown silt, silty sand, and clayey sand seam or lens within the clayey matrix occurs roughly at an elevation of 1075 ft. MSL (varies by location). As mentioned above, the GTU is a continuous lithostratigraphic unit. The transition from Brown Till to Gray Till is gradual. Soil borings did not extend to bedrock, which is anticipated at depths greater than 227 ft. bgs.

Soil boring logs and monitoring well completion logs for the soil borings and monitoring wells shown on Figure 2 are provided in Appendix B, which includes all soil boring and monitoring well logs for investigation activities that were completed in December 2015 and July 2016.

2.3.2 Site Hydrogeology

The movement of groundwater within the geologic formations occurs within the more permeable material (e.g., seams or lenses of clayey sand, silty sand, silt, or sand) within an otherwise fine-grained geologic media (e.g., clay till). Groundwater recharge at the Site is likely from the higher regional water table (north of the Site), precipitation, and seepage from the water retention ponds.

Groundwater Flow

The uppermost aquifer for the Slag Pond Area is the saturated, locally higher-permeability seams and lenses observed within the GTU. Sandy zones reported on boring logs generally do not form correlative horizons. However, a more permeable zone located roughly at an elevation of 1075 ft. MSL frequently appears on logs of adjacent borings.

The water table is defined by the presence of groundwater that is relatively shallow (occurs from 10 to 20 ft. bgs) and is observed in monitoring wells screened across an elevation of ~1075 ft. MSL. The term Upper GTU is used to define the hydrostratigraphy that includes the water table and may include both the Brown Till and Gray Till. The term Lower GTU is used to define the hydrostratigraphy that occurs at elevations below the water table and only includes the Gray Till. Much like the transition from the Brown Till to the Gray Till, the Upper GTU gradually transitions to the Lower GTU, but the transition generally occurs at an elevation lower than the higher permeability seam or lenses at ~1075 ft. MSL.

A downward vertical gradient is apparent when comparing monitoring wells screened at different elevations, especially when comparing wells screened within the Upper GTU and Lower GTU. This phenomena exists because the till forms a natural barrier that restricts the vertical movement of water from shallower beds. These variations in water levels with depth complicate accurate delineation of the water table by making it appear to be highly irregular. This phenomenon is common in till areas and is particularly apparent near groundwater discharge corridors.

Figure 4 shows the temporal groundwater elevations (hydrograph) for monitoring wells included in the monitoring well system, which is described in more detail in Section 4.0. There are currently less groundwater elevation data for monitoring wells installed in July 2016 (SLAG 2B, SLAG 6, SLAG 7, and SLAG 9) than is available for monitoring wells installed in December 2015 (SLAG 4 and SLAG 5). The water level elevations since installation shown on Figure 4 have not fluctuated greatly between readings except for measurements taken at monitoring well SLAG 2B. The fluctuation observed in monitoring well SLAG 2B on September 16, 2016 is believed to be due to the lack of well stabilization from recent well development activities.

Figure 5 shows the groundwater elevations measured on August 8, 2016, which indicates that the water table slopes away from the Cooling Pond towards the south (towards the Whetstone River). Based on the groundwater elevations, groundwater generally flows into the Slag Pond Area from the north and flows south, toward monitoring well SLAG 5.

Laboratory Permeability and Hydraulic Conductivity

Table 1 and Table 2 summarize hydrogeological test results from the Slag Pond Area. Geotechnical laboratory data are available in Appendix C.

Table 1 Slag Pond Area Laboratory Values (GTU)

Boring/ Well	Depth (ft.)	Sample Description	GTU	USCS	Test Type	Hydraulic Conductivity, (cm/s)
SLAG 6	32-34	Clayey Sand w/ trace gravel	Brown	SC	Vertical	6.6x10 ⁻⁷
SLAG 2B	28-30	Clayey Sand w/ gravel and large pockets of sand w/ silt	Gray	SC	Vertical	1.0x10 ⁻⁶
SLAG 2B	28-30	Clayey Sand w/ gravel and large pockets of sand w/ silt	Gray	SC	Horizontal	1.2x10 ⁻⁶
SLAG 9 DEEP	38-40	Sand w/ Silt	Brown	SP-SM	Vertical	1.5x10 ⁻⁴
SLAG 9 DEEP	64-66	Sandy Lean Clay w/ a little gravel	Gray	CL	Vertical	3.7x10 ⁻⁸

Slug test analysis results are summarized in Table 2 and additional details pertaining to the data analysis are included in Appendix D. Slug tests were performed at five monitoring wells (SLAG 1, SLAG 2B, SLAG 5, SLAG 8, SLAG 9 DEEP) to provide estimates of the hydraulic conductivity in the vicinity of the well screens. A slug test consists of monitoring the water-level recovery in a well following an "instantaneous" change in water level. For this work, displacement of the water level in the well was achieved by adding and removing a solid piece of PVC pipe with a known volume. A slug test in which the displacement is initiated by rapidly lowering the slug below the water level is referred to as a slug-in or falling-head test; a slug-out or rising-head test is one in which the slug is rapidly removed. At least two slug tests—slug-in and slug-out—were performed sequentially at each well listed in Table 2. The resulting water-level recovery to static, pre-test condition was monitored using a data-logging pressure transducer (InSitu LevelTroll 700).

Hydraulic conductivity values were estimated using the AQTESOLV software package (Duffield, 2007) to match the Bouwer-Rice (1976) analytical solution against the water-level recovery data. For wells with apparent storage effects (indicated by concave-upward shape in recovery data when plotted on semi-log axes as recommended by Butler (1998) for the Bouwer-Rice solution), aquifer and well construction parameter values required for the analysis were obtained from the available boring logs and well construction records.

Two sets of slug-in/out test pairs were performed at monitoring wells (SLAG 1, SLAG 2B, SLAG 5, and SLAG 8) within the Upper GTU; however the slug-in test data for SLAG 2B were not analyzed due to inadequate recovery during the testing period and the presence of an apparent background trend in the water-level data.

Table 2 Slag Pond Area Slug Test Values (Upper GTU)

Well	Monitored Unit	Hydraulic Conductivity Slug-In (cm/s)	Hydraulic Conductivity Slug-Out (cm/s)
SLAG 1	Water table, upgradient	9.6x10 ⁻⁶	4.0x10 ⁻⁶
SLAG 2B	Water table, downgradient		2.3x10 ⁻⁶
SLAG 5	Water table, downgradient	1.0x10 ⁻⁵	3.8x10 ⁻⁶
SLAG 8	Water table, upgradient	3.6x10 ⁻⁵	5.5x10 ⁻⁵

The horizontal hydraulic conductivity of the Upper GTU monitoring wells shown in Table 2 ranges from 1.0×10^{-5} to 2.3×10^{-6} cm/s based on single-well slug tests, with a geometric mean of 9.4×10^{-6} cm/s. The data shown in Table 2 indicate that the horizontal conductivity (Kh) is roughly the same as the vertical conductivity (Kv) shown in Table 1.

Two sets of slug-in/out test pairs were performed at SLAG 9 DEEP, which is screened approximately at elevation 1055 ft. MSL within the Lower GTU; however, the slug-in test data for SLAG 9 DEEP were not analyzed due to significant noise in the data. The slug-out test for SLAG 9 DEEP indicated a Kh value of 2.2×10^{-6} cm/s, which is slightly lower than the geometric mean value for the horizontal conductivity values provided in Table 2 for wells screened in the Upper GTU. As indicated in Table 1, the Kv value for the clay matrix at SLAG 9 DEEP based on laboratory permeability testing was 3.7×10^{-8} cm/s or over two orders of magnitude less than the Kh in this unit. These data suggest that the Kh is greater than Kv in the Upper GTU, but Kv decreases dramatically in the Lower GTU even though there may be discontinuous seams in the Gray Till at depth that have comparable Kh as the Upper GTU.

Overall, this means there is more resistance to vertical flow in the Lower GTU than the Upper GTU. Based on the data presented above, it is concluded that the GTU becomes less permeable with increasing depth and acts as an underlying confining unit. Therefore the majority of groundwater flow is most likely to occur in the Upper GTU.

Groundwater Flow Rates in the Upper GTU

The rate of groundwater flow is estimated by calculating average linear velocity derived from Darcy's equation:

$$Vt = Kh * i/n = 0.002 ft/day or 0.70 ft/yr$$

Where: Vt = average linear velocity

Kh = hydraulic conductivity $(9.4x10^{-6} \text{ cm/s})$

i = gradient (SLAG 8 to SLAG 5 = 0.018; calculated from water levels)

n = effective porosity (0.25)

The porosity of sand ranges from 0.25 to 0.5 (Freeze and Cherry, 1979). Porosities of glacial outwash aquifers in the region range from 0.2 to 0.3 (Reppe et al, 2005). The assumed porosity of the GTU aquifer is 0.25.

The actual groundwater flow in the vicinity of the Slag Pond Area is likely much lower and is attenuated because the till matrix believed to surround the locally higher permeability seams and lenses will restrict groundwater flow.

Confining Unit Characteristics

The clayey material around the more permeable seams or lenses retard vertical groundwater migration in both of the GTU till units. Based on the slug tests and laboratory tests, the Lower GTU appears to be a confining unit relative to the Upper GTU. This is because the distribution of hydraulic conductivity appears to be more strongly anisotropic (e.g., directional because Kh>>Kv) in the Lower GTU than the Upper GTU, which suggests a greater resistance to vertical flow. Anisotropy is common in horizontally-deposited strata and would tend to result in a natural barrier to the downward vertical movement of water below the Upper GTU.

2.4 Potential Groundwater Receptors

Barr (1990) evaluated information on area wells and found that there are two water supply wells in the vicinity of the Slag Pond related to an ethanol plant (POET Biorefinery) well and the Plant supply well (1977 well log). A third well, called the Champion well, is located between the Slag Pond and the Brine Ponds, but is not used for potable water supply. These wells are not screened within the Upper GTU and are separated from the uppermost saturated unit by a thick layer of low-permeability clay till, and therefore are unlikely to be affected by the Slag Pond Area. The downgradient receptor of groundwater from the Slag Pond Area is the Whetstone River.

2.5 Well Development

Well development was completed to remove fines from the water column in the sand pack adjacent to the well screen and to improve formation permeability near the borehole that may have been influenced by drilling activities. Monitoring wells were surged several times initially by raising the pump up and down within the casing to settle the sand pack and collapse voids in the filter pack caused by bridging. Monitoring wells identified to be within the monitoring well system discussed in Section 4.0 were developed by a combination of higher-rate pumping followed by low-volume pumping without significant surging.

Volumes of purge water removed, relative clarity, and turbidity were measured at each well during development. Well development continued until the water from the well was relatively sediment free, appeared clear, and had decreasing trends in turbidity measurements. Table 3 provides the approximate lowest obtained turbidity measurement, total volumes purged, and the approximate well recharge rates for each well developed.

Table 3 Turbidity, Purge, and Recharge Field Measurements

Well ID	Lowest Obtained Turbidity Measurement (NTU)	Approx. Total Amount Volume Purged (gal)	Арргох. Most Recent Recharge Rate (ft.) [date]
SLAG 2B	NA	NA	NA
SLAG 4	7	24	18 minutes to recharge 4 ft. [8/8/16]
SLAG 5	15	4.3	35 minutes to recharge 6 ft. [8/8/16]
SLAG 6	15	15	20 minutes to recharge 5 ft. [8/8/16]
SLAG 7	29	29	3 hours to recharge 24 ft. [8/8/16]
SLAG 8	7	13.3	30 minutes to recharge 17 ft. [8/8/16]
SLAG 9	38	90	1 hour to recharge 24 ft. [8/8/16]

NA – Not Available

Table 3 also shows the approximate recharge rate measured by pumping the well dry and then measuring its recovery. Turbidity, purge, and recharge field measurements for monitoring well SLAG 2B were conducted by OTP.

3.0 Conceptual Models

3.1 Slag Pond Area Conceptual Model

Cross section locations for the Slag Pond Area are shown on Figure 6 and include the location of cross section A-A', B-B', C-C', and D-D'. Cross section A-A' is shown on Figure 7; cross section B-B' is shown on Figure 8; cross section C-C' is shown on Figure 9, and cross section D-D' is shown on Figure 10. The groundwater elevations shown on Figure 7 through Figure 10 were collected on August 8, 2016.

In summary, Figure 7 through Figure 10 show the following features about the hydrogeology of the Slag Pond Area:

- The surface materials consist of the Brown Till.
- As mentioned in Section 2.3.2, uppermost aquifer groundwater flow is primarily within the more permeable zone located roughly at an elevation of 1075 ft. MSL that frequently persists between adjacent borings.
- The water table surface slopes approximately between 1080 ft. to 1110 ft. MSL as observed in monitoring wells.
- The boring log for SLAG 2B shows the Gray Till was observed at approximately elevation 1085 ft.
 MSL, which is higher than observed in other soil borings completed along the south side of the Slag Pond Area (Appendix B).
- The pilot boring completed along SLAG 8 shows the Gray Till was observed at approximately elevation 1085 ft. MSL, which is higher than observed in other soil borings completed along the north side of the Slag Pond Area (Appendix B).
- Groundwater and surface water measurements indicate that shallow groundwater flows from the Cooling Pond towards the Slag Pond Area and onward, presumably to the Whetstone River approximately one mile south of the Slag Pond Area.
- A downward vertical gradient is apparent when comparing monitoring wells screened within the Upper GTU and Lower GTU (see water level shown for SLAG 9 DEEP on Figure 7).
- The transition from the Upper GTU to the Lower GTU is gradual and occurs at an elevation lower than the higher permeability seam or lenses at ~1075 ft. MSL.
- Soil borings completed at the Site in December 2015 and July 2016 extended to a maximum depth of 68 ft. bgs

3.2 Release Conceptual Model

A release conceptual model uses the groundwater flow direction and geologic information of the site conceptual model to predict the likely pathway of a release from a CCR unit to groundwater so that a monitoring system can be positioned properly to intercept it.

A hypothetical release at the Slag Pond Area would likely be transported southeast, the downgradient direction of the water table shown on Figure 5. The downgradient wells discussed in the next section are positioned to ensure detection of any contaminants from such a release.

4.0 CCR Groundwater Monitoring System

Figure 11 shows and Table 4 describes the CCR groundwater monitoring system for the Slag Pond Area.

Table 4 Monitoring Well System Summary

Well ID	Well Placement	Rationale
SLAG 8 and SLAG 9	Upgradient	To account for hydrogeologic variability upgradient of the Slag Pond Area and to establish a sufficient number of upgradient monitoring wells at appropriate locations and depths to yield groundwater samples of the uppermost aquifer not impacted by the CCR unit (257.91(a) (1) and (2)).
SLAG 2B, SLAG 4, SLAG 5, SLAG 6 and SLAG 7	Downgradient	To detect a release from the Slag Pond Area and to account for hydrogeologic variability, establish sufficient number of downgradient monitoring wells at appropriate locations and depths to yield groundwater samples of the uppermost aquifer accurately representing the quality of groundwater passing through the waste boundary (257.91(a) (1) and (2)).

Based on our observations during sampling and well-development activities, the upgradient and downgradient monitoring wells included in the monitoring system are designed and constructed to provide representative groundwater samples. Based on the monitoring well completion logs available, each well has a casing that is screened; the annular space between the screen and borehole is filled with sand and the annular space above the sand pack is sealed. The downgradient wells listed in Table 4 are positioned to detect contaminants from a hypothetical release in the Slag Pond Area.

In summary, the groundwater monitoring system identified in Table 4 and on Figure 11 is deemed to be adequate for groundwater monitoring under the CCR Rule requirements. Table 5 provides construction details of the CCR groundwater monitoring wells.

Table 5 CCR Monitoring Well Details

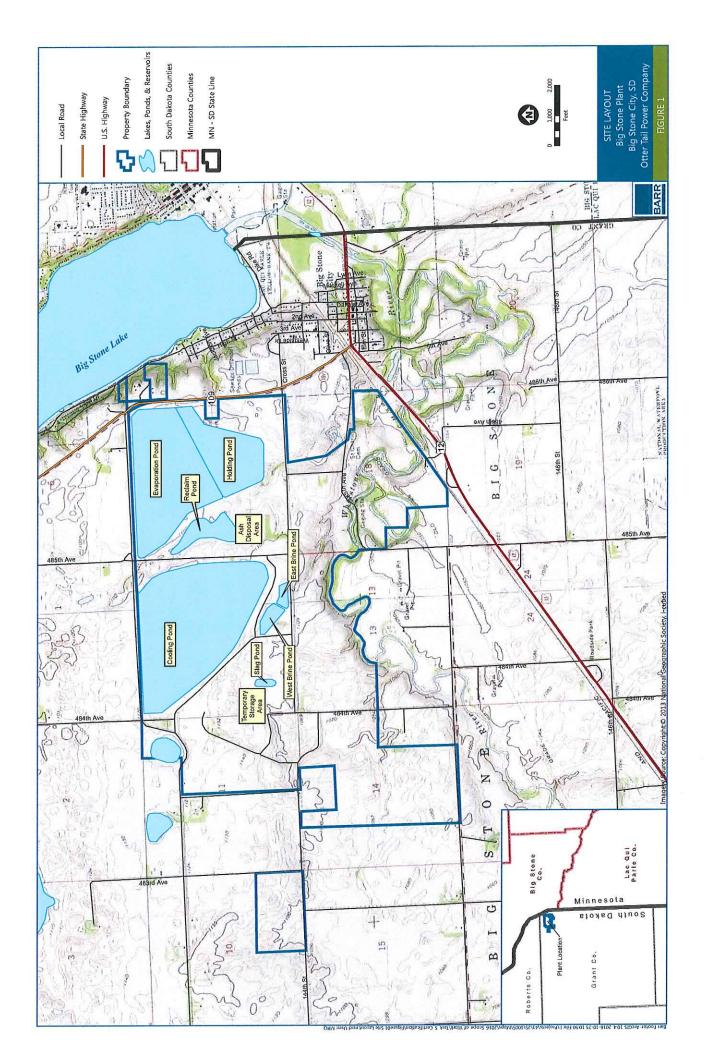
Well	Installation Date	TOR (ft. MSL)	Total Depth BMP (ft.)	Screen Length (ft.)/ Diameter (in)	Casing/Screen/Slot
SLAG 2B	7/31/2016	1111.26	40.25	5/2.0	PVC/PVC/#6
SLAG 4	12/23/2015	1,113.15	39.02	10/2.0	PVC/PVC/#6
SLAG 5	12/23/2015	1,107.30	38.8	10/2.0	PVC/PVC/#6
SLAG 6	7/20/2016	1111.51	37.3	10/2.0	PVC/PVC/#6
SLAG 7	7/21/2016	1114.39	48.7	10/2.0	PVC/PVC/#6
SLAG 8	NA	1127.26	52.1	5/2.0	PVC/PVC/#10
SLAG 9	7/28/2016	1122.35	49.8	10/2.0	PVC/PVC/#6

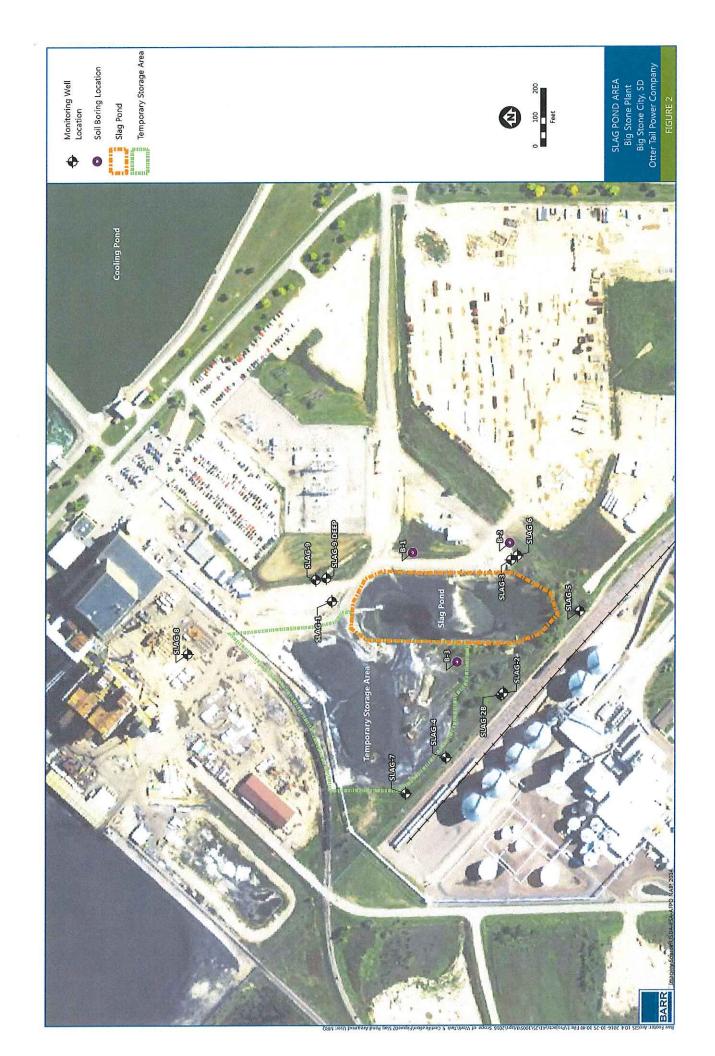
NA = Not Available

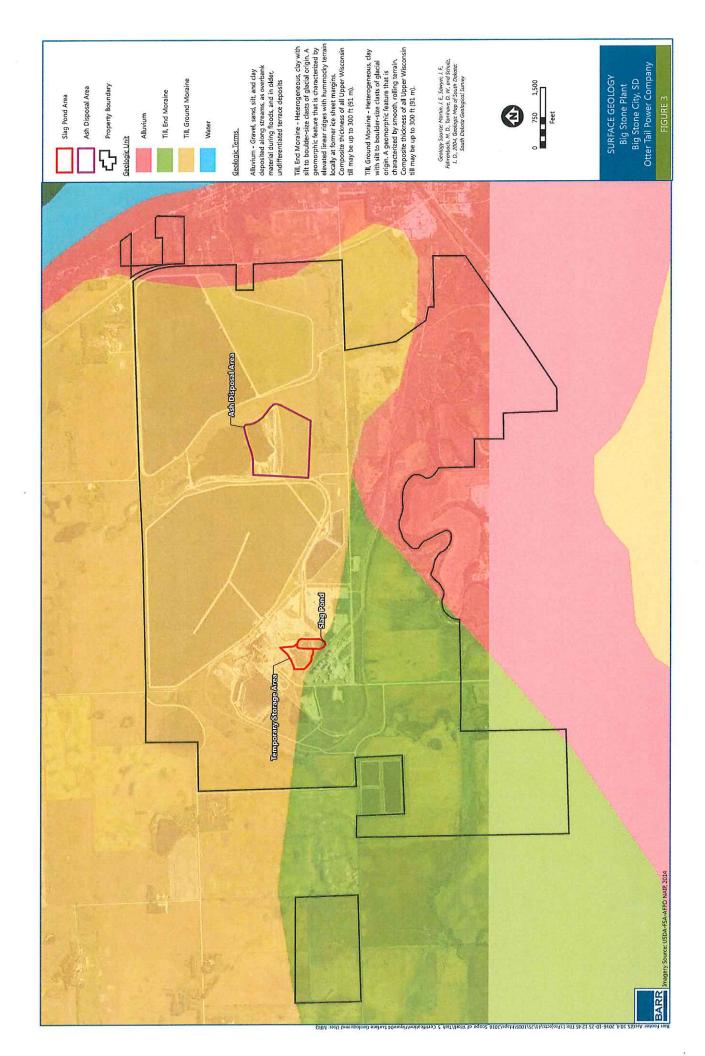
5.0 References

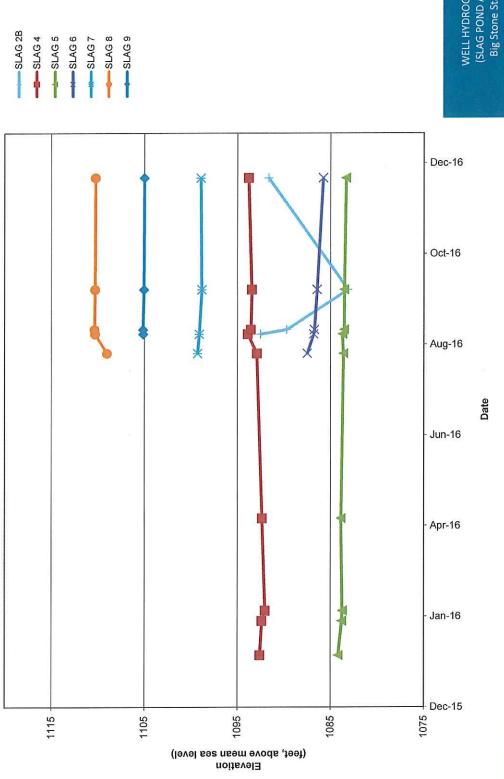
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Figures





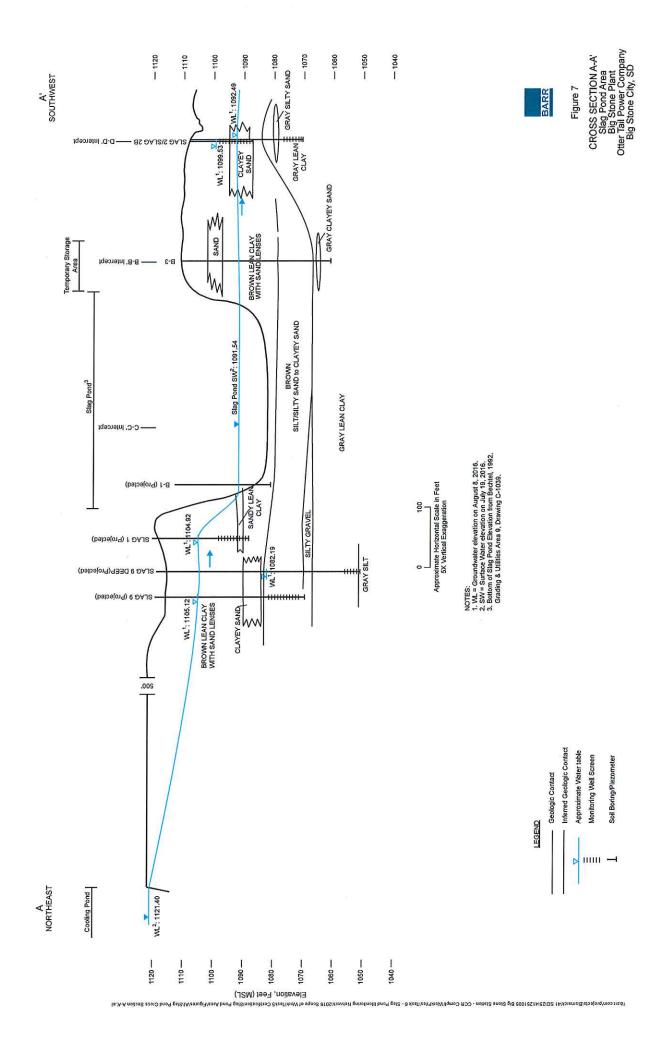


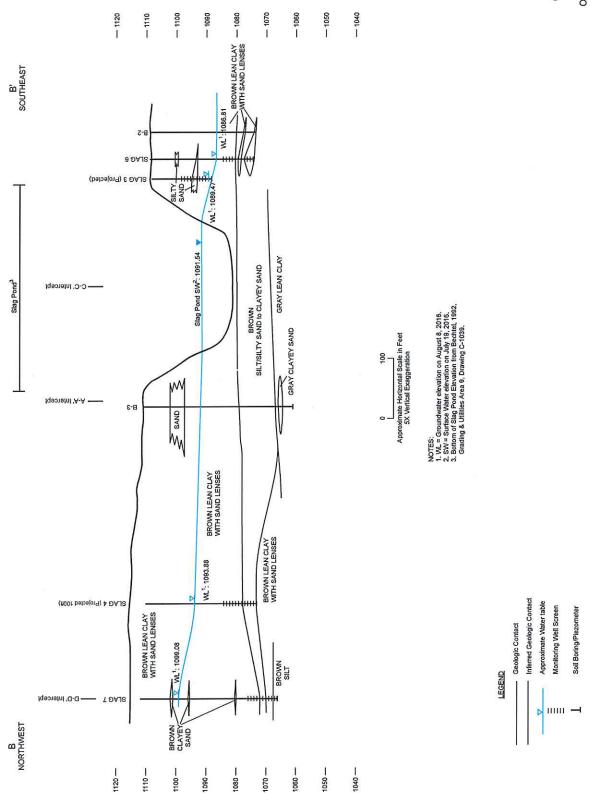




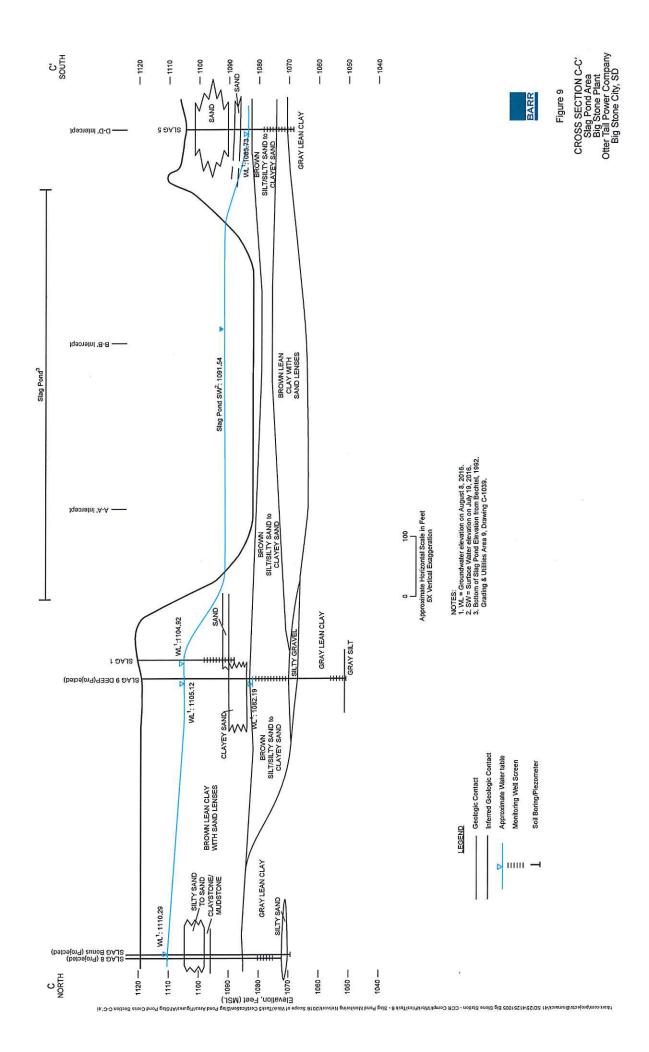


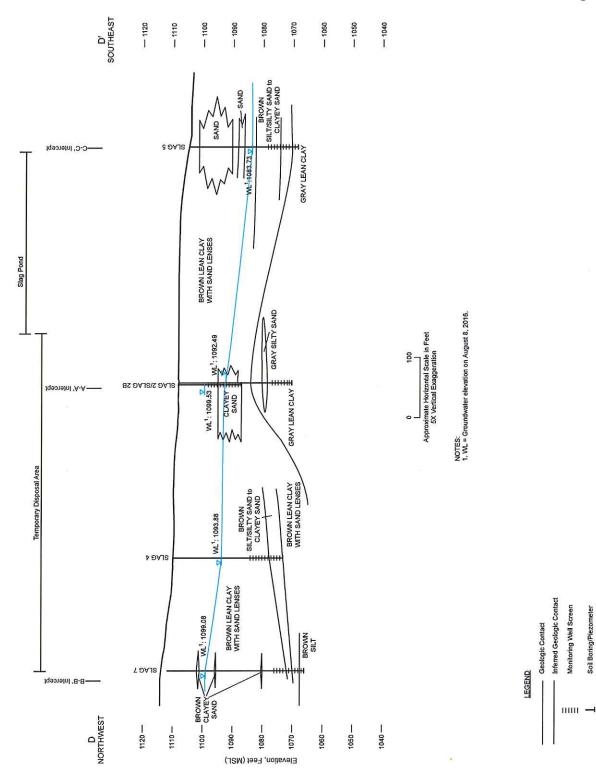






Elevation, Feet (MSL)







Appendices

Appendix A

1977 Boing Log

121-47-12 CDAD ?



LAYNE MINNESOTA COMPANY

3147 CALIFORNIA STREET, N.E. MINNEAPOLIS MINNESOTA

WELL LOG

JOB NA	ME Ott	certail Powe	er Company			5	TARTED	, 19		
							OMPLETED <u>Auqust</u>			
17 -			7-46-W				OB NUMBER	According to a second second		
			ORMATI				0 0	1114'		
FROM	то	MATERI		FR	ом	то	MATERIA	L		
20	32 Br	llow silty . clayey sa	ind	1.3	0 1	50	Blue sand with o	clav streak		
32 40	51 Br	. coarse sa . clay		1.5	0 1	100	Blue sand clean			
57	93 B	ue clay ue medium f ue clayey s	ine sand	17	7 2		Blk. shale w/gra	ivel &		
73 75	90 Br	ue clayey s . sand (dir	and (Ey)				lignite ler	The same of the sa		
90		k. clay wit		,22	7 2	52	White coarse sar	d		
METHOD	05 0011	tite mil	010 404	D14		n		d 17"		
							METER OF HOLE			
							BUNT OF CEMENT B GI			
							asing254 FT. Stati			
ONDER I	REAMED FI	9E mm + 5	70 252 FT. D				Метнор <u>Hydraulic</u>			
		85 mm. to					AMOUNT2	The state of the s		
WELL 5	40T AT	FT. TO	FT. NUMBER		SIZE		LBS. REMOVED	C.A.		
	12.00									
	MAT	ERIAL	INSTAL	LED		IN	WELL			
-	OPENING	LENGTH	Name and Add to the Owner, or the Owner, where the Owner, which is the Owner, which is the Owner, where the Owner, which is the Own	LAMETE	-		MATERIAL			
SCREEN		25'	"	6 "	Name and Address of the Owner, when the Owner, which the Owner	Sta	tainless steel			
DRIVE C	ASING	227	. н	12 "		Stee	teel			
LINER C	ASING	201	18	6 "		Sta	tainless steel riser pipe			
				-		<u></u>				
		PUMP	ING T	r E s ·	t [*]					
HRS PUM	PED	- J. P. P. T.	WATER LEVEL		ORAWIN	own	REMARK			
FROM	то	ALETD	BELOW SURFAC				33. 32. 11 H. W. IV.			
		GPM		11	,	11	1			
		GPM			,		'			
		GPM	1	n						
		GPM		"						
	E		a a market							
	CLEAR						TYGPM/FT	OF DD.		
ATE		, 19	DRILLER	Neil	Rol	lie				

Layne minnesota company

3147 CALIFORNIA STREET, N.E. MINNEAPOLIS MINNESOTA Permit #1982-3 8 SESW 12 T1211

Permit #1982-3 & 4881-3 SESW 12 T121N R47W

JOB Ottertail Power Company

LOCATION Big Stone Plant

22

WELL NO. 77-1 STATIC LEVEL 88' DATE August 23, 19 77

8/22			WATER	WATER		PPM		
	2:00pM	4"	234	190 '	102'	.5		Mostly Shale
	3:00PM	4"	247	192'	104'	. 3		Sand, some shale
	4:00PM		239	193'	105'	. 1		Pieces of shale
A section to the latest the section to the section	5:00PM	4"	243	191'	103'	.0		Pieces of shale
	6:00PM	4"	243	191'	103'	,0		Traces of shale
	7:00PM	4"	243	192'	104'	.0		Traces of shale
	8:00PM	4"	239	193'	105'	.0		Traces of shale
	9:00PM	4"	230	193 '	105'	.0		Traces of shale
	10:00PM	4"	234	195'	107!	.0		Traces of shale
	11:00PM	4"	230	195'-6"	107'-6"	.0		Traces of shale
	12:00	4"	225	195'-6"	107'-6"	.0		Traces of shale
8/23	1:00AM	4"	230	195	107'-6"	.0		Traces of shale
	2:00AM	4"	230		108'	.0	2	Traces of shale
	3:00AM	4"	230	195'-6"	107'-6"	.0		Traces of shale
	4:00AM	4"	225	196'	L08,	.0		Traces of shale
	5:00AM	4"	225	195'-6"	107'-6"	.0		Traces of shale
	6:00AM	4"	225	195'-6"	107'-6"	.0		Traces of shale
	7:00AM	4"	225	195'-6"	1.07'-6"	0		Traces of shale
	8:00AM	4"	220	195'-6"	107'-6".	0		Traces of shale
	9:00AM	4"	205	195'-6"	107'~6"	.0		Traces of shale
	10:00AM	4"	210	194'	1.06	.0	, and the second	Very small piece
	11:00AM	4"	200	180'	92'	.0		Bigger pieces sh
	12:00	4"	200'	183'-6"	195'-6"	.0		Very small piece
	1:00PM	4"	200	182'	94'	.0	All	
	2:00PM	4"	200	182'	94 '	.0		

REMARKS	10:45	AM,	8/23,	/77,	water	became	ver	су с	loudy,	didn't	clean	nb	until	10:45
	Lots	of sl	nale.			-							1	territoria finalisticational
DATE:		8/3	23	, 19 77	DRIL	LER: N	eil	Ro1	lie					

LAYNE MINNESOTA COMPANY

3147 CALIFORNIA STREET, N.E. MINNEAPOLIS MINNESOTA Permit #1982-3 & 4881-3
SESW 12 T121N R47W
RECOVERY

JOB Ottertail Power Company

LOCATION Big Stone

EMARKS

WELL NO. #1 STATIC LEVEL 88' DATE August 23 ,1977

DATE	HOUR	ORIF.	GPM	DEPTH TO	DRAWDOWN	SANO	GPM/FT.	REMARKS
			WATER	WATER		PPM	3,1711.	NEMBRAS
8/23	2:00PM		200	182'				Shut off
	2:01PM		200	116'				
	2:02PM			104'				
	2:04PM			102'-5"				
-	2:06PM			101'				
***	2:08PM			100'				
	2:LOPM			100'				
	2:15PM			99'-6"				
	2:20PM			99'-6"				
	2:30PM			991				
-	2:40PM			991				
-	2:50PM			98'-8"				
	3:00PM			98'-4"				
	3:30PM			98'				4.150
	5:30PM			97'				
	7:30PM		-	96'-6"				
3/24	12:00'					-		
			-					
	1 - v							
-								
	-							
****				-				· ·

DATE:	August	23,19_77	DRILLER:	Neil	Rollie	

AYNE MINNESOTA COMPANY

BLAT CALIFORNIA STREET, N.E. MINNEAPOLIS MINNESOTA





Permit #1982-3 & 4881-3 SESW 12 T121N R47W

JOB Ottertail Power Company

LOCATION Big Stone Plant

WELL NO Observation WeldTATIC LEVEL 89' DATE August 23,19 77 200' Distant

DATE	HOUR 1	ORIF.	GPM WATER	DEPTH TO WATER	DRAWDOWN	SANO PPM	GPM/FT.	REMARKS
8/22	2:00PM			90'-6"				
	3;00PM			91!				(
	4:00PM		- VI	921				
	5:00PM			941				
	6:00PM			941				
	7:00PM			94 '			A	
	8:00PM			94 '				
	9:00PM			95'-6"				
	10:00PM			95'-6"				
	11:00PM			96'-0"				
	12:00			96'-6"				
8/23	1:00AM			97'-6"				
	2:00AM	START MARKS		98'				
	3:00AM			97'-6"				
	4:00AM			97'-6"			-1.1Mm/1.00	
	5:00AM			981				
	6:00AM			98'-6"				
	7:00AM			991				
	MA00:8			991				
	9:00AM			99!		***************************************		
	10:00AM			1.00 '				
	11:00AM			100'				
	12:00	,		100'				
	1:00PM			100'		-10-20-20		· · · · · · · · · · · · · · · · · · ·
	2:00PM			100'				
		- LANGE BOOK - LAN						
			p. 44					
		V 100						

REMARKS_					
DATE:	August	23,1977	DRILLER:	Neil Rollie	-

YNE MINNESOTA COMPANY 3147 CALIFORNIA STREET, N.E.

MINNEAPOLIS



Permit #1982-3 & 4881-3 SESW 12 T121N R47W

RECOVERY

JOB Ottertail Power Company

LOCATION Big Stone Plant

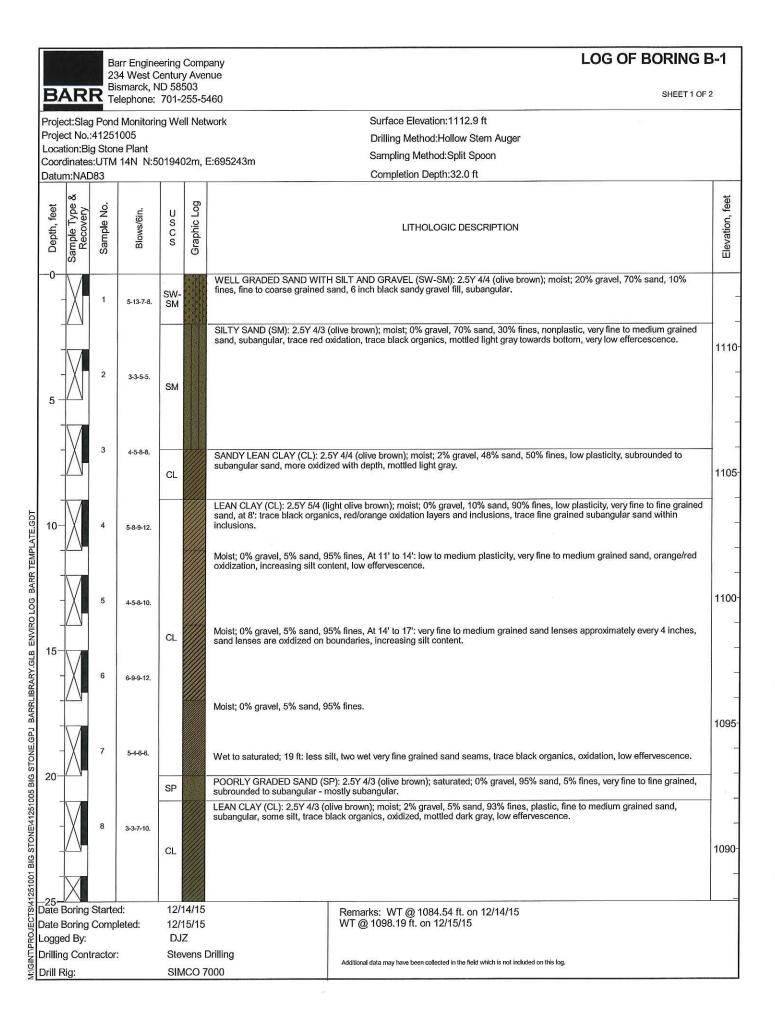
WELL NO. Observation STATIC LEVEL 89' DATE August 23 ,1977

DATE	HOUR *	ORIF.	GPM WATER	DEPTH TO WATER	NWOOWA RD	SAND PPM	GPM/FT.	REMARKS
8/23	2:00PM			100'				Shut off
-	2:01PM			99'-6"				
	2:02PM			99'-4"	,			
	2:04PM			99'-4"				
	2:06PM			99'-2"				
	2:08PM			99'-0"				
	2:10PM			98'-9"				
	2:15PM			98'-9"		5		
	2:20PM			98'-5"				
	2:30PM		5 min	98'-4"	A CONTRACTOR OF THE CONTRACTOR			
	2:40PM			98'-4"				
	2:50PM			98'-1"				
	3:00PM			98'-0"		- 17		
	3:30PM			97'-9"				
	5:30PM			97'-8"				
	7:30PM			97'-3"				
8/24	12:00			961				
					-			
								The state of the s
								-Post-representative acquire conditional Review
		70 -						
			= = = =					

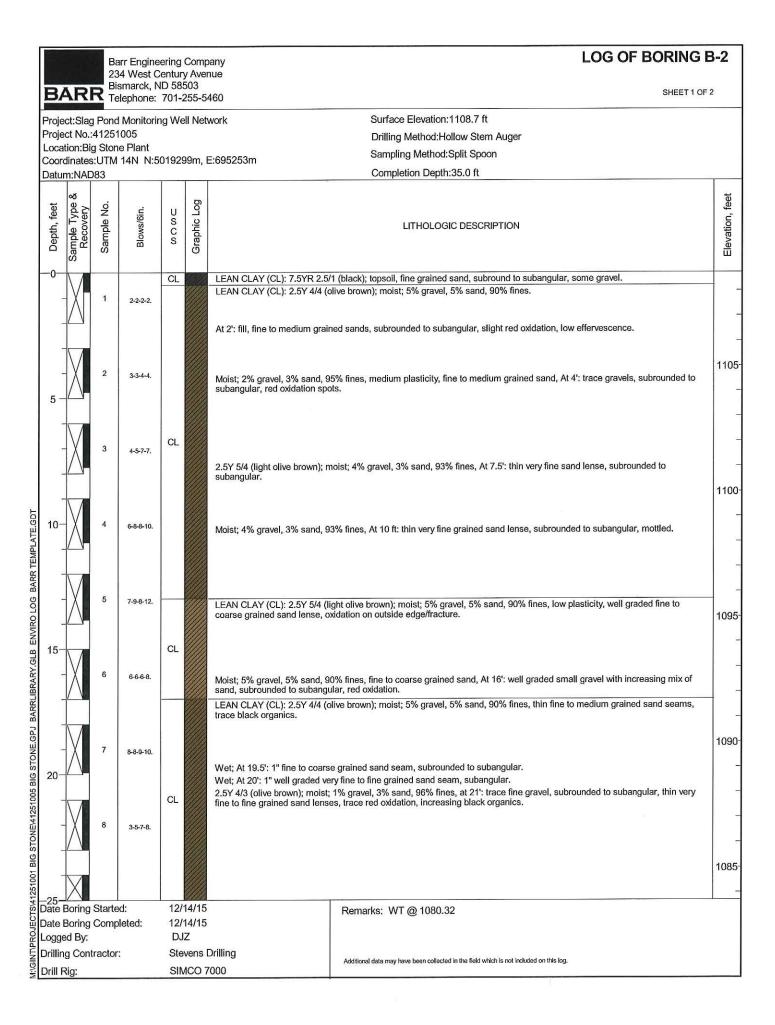
(K. 1/=					
DATE:	August	23,19,77	DRILLER:	Neil Rollie	

Appendix B

Soil Boring Logs (Slag Pond Area)



982		arr Engine				LOG OF BORING B-	1
BAR	Bi	34 West C smarck, N elephone:	D 58	503		SHEET 2 OF 2	
Project:Sla Project No. Location:B Coordinate Datum:NAI	:4125 ig Stor s:UTM	1005 ie Plant			vork E:695243m	Surface Elevation:1112.9 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:32.0 ft	
Depth, feet Sample Type & Recovery	Sample No.	Blows/6in.	U S C S	Graphic Log		LITHOLOGIC DESCRIPTION	Elevation, feet
-25	9	4-10-12-10.	CL		LEAN CLAY (CL): 2.5Y 4/3 (olive subangular, some silt, trace black	brown); moist; 2% gravel, 5% sand, 93% fines, plastic, fine to medium grained sand, corganics, oxidized, mottled dark gray, low effervescence. (continued)	
	10	6-7-8-11.	SP- SM		POORLY GRADED SAND WITH	SILT (SP-SM): wet; 0% gravel, 90% sand, 10% fines, very fine grained sand with silt.	1085
30	11	6-7-13-21.	CL		medium grained sand, subrounde	orounded and subangular sand with silt and oxidized layers, lense of well graded subangular	
35-						, e	
40-						· · · · · · · · · · · · · · · · · · ·	
35— - 40— - 45— - Date Boring Date Boring Logged By: Drilling Cor Drill Rig:							
Date Boring Date Boring Logged By:	g Starte g Comp	ed: oleted:		14/15 15/15 Z	F V	Remarks: WT @ 1084.54 ft. on 12/14/15 VT @ 1098.19 ft. on 12/15/15	
Drilling Cor Drill Rig:	itractor	7		vens I ICO 7	Orilling 000	additional data may have been collected in the field which is not included on this log.	



	'Y''		rr Engine				-2
BA	۱RI	Bis	4 West C smarck, N lephone:	D 58	503	SHEET 2 OF 2	
Proje Loca Coore	ct No.: tion:Big	41251 g Ston s:UTM	e Plant			vork Surface Elevation:1108.7 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:35.0 ft	
Depth, feet	Sample Type & Recovery	Sample No.	Blows/6in.	Uscs	Graphic Log	LITHOLOGIC DESCRIPTION	Elevation, feet
25- -	X	9	4-6-8-9.	CL		LEAN CLAY (CL): 2.5Y 4/4 (olive brown); moist; 5% gravel, 5% sand, 90% fines, thin fine to medium grained sand seams, trace black organics. (continued) Moist; 2% gravel, 3% sand, 95% fines, AT 25': increased very fine to fine grained sand content, increasing moisture, oxidized orange and red layers, low effervescence.	
		10	3-5-9-11.	CL		LEAN CLAY (CL): 2.5Y 5/3 (light olive brown); moist; 0% gravel, 2% sand, 98% fines, trace very fine grained sand, subrounded to subangular, oxidized orange and red layers, trace black organics, low effervescence.	
30-				sc		CLAYEY SAND (SC): wet; 0% gravel, 80% sand, 20% fines, very fine to fine grained sand, subrounded to subangular, trace silt and clay, some oxidation, black spots, very low effervescence.	1080
27		11	7-7-7-9.	CL		LEAN CLAY (CL): 10YR 3/1 (very dark gray); wet; 6" layer mottled with fine to medium grained sand. CLAYEY SAND (SC): 0% gravel, 85% sand, 15% fines, some areas layered fine grained sand/clay.	
85		12	6-7-9-11.	sc			1075
35-			-57	⟨CL/		SANDY LEAN CLAY (CL): 10YR 3/1 (very dark gray); wet; very fine grained oxidized sand pockets within. End of boring 35.0 feet	(-
ä						~	
7.C							
40-	-						
2.						un.	
45-	-						
40- Joate Logg Drilling							
-50- Date Date	Boring Boring				 14/15 14/15	Remarks: WT @ 1080.32	
Logg Drillir	e Boring Completed: 12/14/15 ged By: DJZ ing Contractor: Stevens Drilling Rig: SIMCO 7000					Additional data may have been collected in the field which is not included on this log.	

Cole		arr Engin				LOG OF BORING	B-3
BARF	D.	34 West (smarck, elephone	NID F	0500		SHEET 1 O	F2
Project:Slag Project No.: Location:Big	Pon 4125 Sto :UTN	d Monito 1005 ne Plant	ring \	Well N		Surface Elevation:1111.1 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:50.0 ft	
Depth, feet Sample Type & Recovery	Sample No.	Blows/6in.	USCS	Graphic Log		LITHOLOGIC DESCRIPTION	Elevation, feet
0	1	4-5-5-6.			LEAN CLAY (CL): frozen; tra	ce topsoil, black ash runoff.	1110
	2					noist; 0% gravel, 10% sand, 90% fines, At 2': low plasticity, very fine to fine with trace medium subangular, larger grains subangular, trace black vitreous bottom slag, low effervescence.	,
5	2	3-3-3-4.	CL			gravel, 2% sand, 98% fines, At 4": 6": trace oxidation, roots/ organics, no effervescence.	
	3	5-5-6-8.			2.5Y 5/4 (light olive brown); 2 grained sand, subrounded to	% gravel, 8% sand, 90% fines, mottled with 2.5y 7/1 (light gray), very fine to fine with trace medium subangular, larger grains more subangular, oxidation, none to low effervescence.	1105
10-	4	6-8-7-8.			POORLY GRADED SAND (S grained sand, subrounded to layering, no effervescence.	P): 2.5Y 5/4 (light olive brown); moist; 0% gravel, 95% sand, 5% fines, very fine with some medium subangular, larger grains more subangular, trace red oxidation, little black spots within, some	
	5	4-7-7-6.	SP		0% gravel, 95% sand, 5% fine	es.	1100
15						gravel, 10% sand, 85% fines, medium plasticity, very fine to medium grained sand, subrounded to ravel, some dark brown/black oxidation, trace black organic chunks.	-
	6	7-7-9-12.	CL				1095
20-	7	3-5-7-10.			Moist; 0% gravel, 10% sand,	s 2 33 13 (cons 2 3 4)	-
	8	4-4-6-7.	CL			gravel, 10% sand, 90% fines, medium plasticity, very fine to medium grained sand, subrounded to e subangular, trace black organics, some oxidation in thin very fine grained sand layers.	1090
layering, no effervescence. SP						gravel, 5% sand, 95% fines, increasing silt content, very fine grained sand lenses to larger fine bangular sand, red and black oxidation.	-
Date Boring S Date Boring C Logged By:	Date Boring Started: 12/15/15 Date Boring Completed: 12/15/15 Logged By: DJZ					Remarks:	-
Drilling Contra Drill Rig:	rilling Contractor: Stevens Di rill Rig: SIMCO 70					Additional data may have been collected in the field which is not included on this log.	

LOG OF BORING B-3 Barr Engineering Company 234 West Century Avenue Bismarck, ND 58503 BARR Telephone: 701-255-5460 SHEET 2 OF 2 Project:Slag Pond Monitoring Well Network Surface Elevation:1111.1 ft Project No.:41251005 Drilling Method:Hollow Stem Auger Location:Big Stone Plant Sampling Method:Split Spoon Coordinates:UTM 14N N:5019355m, E:695127m Completion Depth:50.0 ft Datum:NAD83 Graphic Log Depth, feet Sample Type Sample No. U Blows/6in Elevation, SCS LITHOLOGIC DESCRIPTION 1085 LEAN CLAY (CL): moist; 0% gravel, 5% sand, 95% fines, increasing silt content, very fine grained sand lenses to larger fine grained to medium grained subangular sand, red and black oxidation. (continued) 10 4-4-5-9 CL 30 11 1080 2-3-5-5. SILTY SAND (SM): at 31.5': 6" thick. SM : CLAYEY SAND (SC): 2.5Y 4/4 (olive brown); saturated; 0% gravel, 50% sand, 50% fines, transitioning. SC 12 3-3-4-4 SILTY SAND (SM): 2.5Y 4/4 (olive brown); saturated; no effervescence. 35 SM 1075-CLAYEY SAND (SC): 2.5Y 4/4 (olive brown); very fine grained sand. SC 13 CLAYEY SAND (SC): moist; 0% gravel, 50% sand, 50% fines, At 37.5': very fine to medium grained sand, subrounded to subangular, coarse grains more subangular, some dark brown/black oxidation - manganese, some saubangular gravels, trace black organic chunks. 2.5Y 4/3 (olive brown); moist; medium plasticity, very fine to medium grained sand, subrounded to subangular, larger grains more subangular, oxidation, trace black organics, low effervescence. SC 14 4-4-6-8. 1070-2.5Y 4/3 (olive brown); moist; medium plasticity, subrounded to subangular sand, larger grains more subangular, oxidation, trace black organics, low effervescence. CLAYEY SAND (SC): 2.5Y 4/3 (olive brown); saturated; 0% gravel, 50% sand, 50% fines, very fine to fine grained sand, no effervescence, soupy. 15 SC 7-13-21-24. LEAN CLAY (CL): 2.5Y 3/1 (very dark gray); moist; medium plasticity, medium grained sand, 6", subrounded, oxidation. CL 2.5Y 4/3 (olive brown); moist; medium plasticity, very fine to fine with trace medium grained sand, subrounded to subangular, arger grains more subangular, 9", oxidation, trace black organics, low effervescence. SC 1065 11-9-9-12. CLAYEY SAND (SC): wet; 3" sand seam, very fine to fine grained sand, subrounded to subangular. SANDY LEAN CLAY (CL): 2.5Y 3/1 (very dark gray); At 46.5': very fine to medium grained sand, subangular, soft, low effervescence. CL 17 6-7-10-10. LEAN CLAY (CL): 2.5Y 3/1 (very dark gray); moist; 0% gravel, 5% sand, 95% fines, At 49.5': low plasticity, very fine to fine grained sand, mostly subrounded, no oxidation, soft, low effervescence. 2.5Y 4/1 (dark gray); medium plasticity. CL End of boring 50.0 feet 12/15/15 Date Boring Started: Remarks: 12/15/15 Date Boring Completed: DJZ Logged By: Stevens Drilling **Drilling Contractor:** Additional data may have been collected in the field which is not included on this log. **SIMCO 7000** Drill Rig:

BARR TEMPLATE.GDJ

ENVIRO LOG

BIG STONE/41251005 BIG STONE.GPJ BARRLIBRARY.GLB

251001

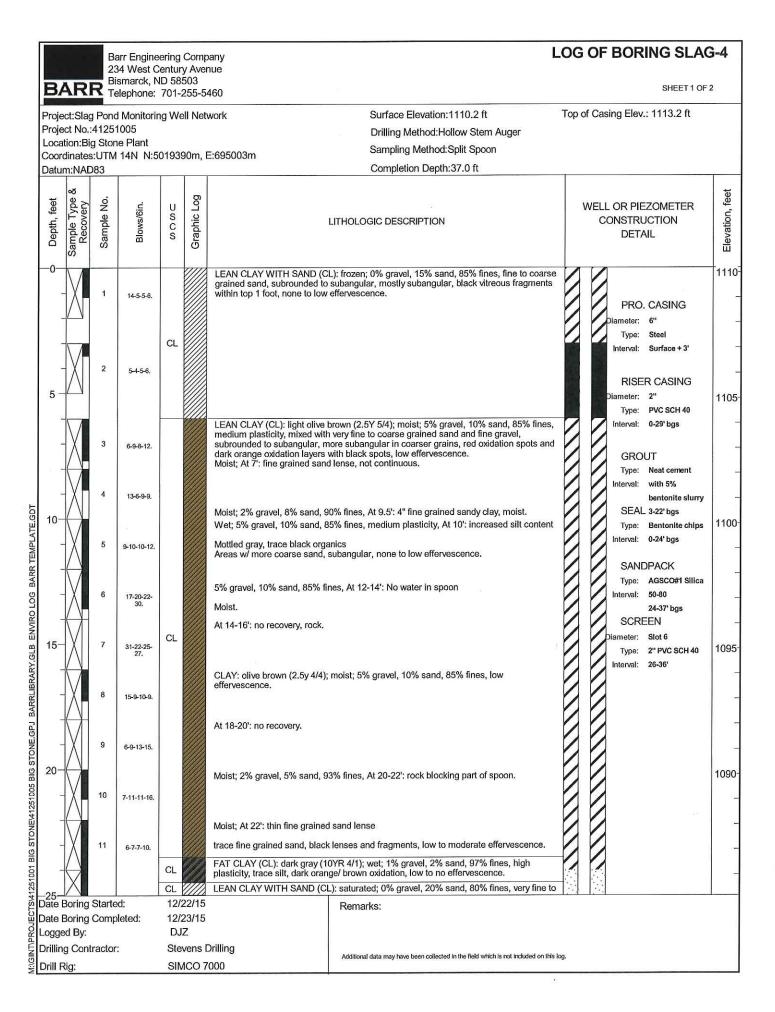
LOG OF BORING SLAG-1 Barr Engineering Company 234 West Century Avenue Bismarck, ND 58503 SHEET 1 OF 2 BARR Telephone: 701-255-5460 Top of Casing Elev.: 1123.0 ft Surface Elevation:1120.3 ft Project:Slag Pond Monitoring Well Network Project No.:41251005 Drilling Method: Hollow Stem Auger Location:Big Stone Plant Sampling Method:Split Spoon Coordinates: UTM 14N N:5019487m, E:695191m Completion Depth:32.3 ft Datum:NAD83 Elevation, feet Sample Type & Recovery Graphic Log Depth, feet Sample No. WELL OR PIEZOMETER USCS Blows/6in LITHOLOGIC DESCRIPTION CONSTRUCTION DETAIL SANDY LEAN CLAY (CL): 2.5Y 4/3 (olive brown); frozen; 10% gravel, 30% sand, 60% fines, fine to medium grained sand, subrounded to subangular, few subangular gravel. 1120 30-9-10-10. PRO. CASING CL meter: 6" Type: Steel Interval: Surface + 3' LEAN CLAY (CL): wet; 2% gravel, 5% sand, 93% fines, low plasticity, fine to medium grained sand, subrounded to subangular, trace subangular gravel, around 5' most likely bottom of a ditch & both sides of well, none to low effervescence. 2 21-14-18-RISER CASING 5 1115-Type: PVC SCH 40 CL Interval: 0-25' bgs Medium plasticity, Wet at 6' bgs. 6-5-5-7. GROUT Type: Neat cement with 5% LEAN CLAY (CL): 2.5Y 5/4 (light olive brown); moist; 0% gravel, 5% sand, 95% fines, medium plasticity, very fine to fine with trace medium grained sand, well graded, subrounded to subangular - mostly subangular, few sand lenses toward bottom, some bentonite slurry SEAL 3-17.5' bgs BARR TEMPLATE.GDJ 4-5-8-9. Type: Bentonite chips 1110lenses oxidized on boundaries, trace red oxidized spots, low effervescence. Interval: 0-20' bgs SANDPACK Type: AGSCO#1 Silica Interval: 50-80 **ENVIRO LOG** 5-4-5-9 20-32' bgs **SCREEN** Moist; 1% gravel, 4% sand, 95% fines, at 14': thin (less than 1/4") sand lense. Slot 6 Diameter: Moist; 2% gravel, 4% sand, 94% fines, At 15': no sand lense, trace oxidation, no BIG STONE/41251005 BIG STONE.GPJ BARRLIBRARY.GLB Type: 2" PVC SCH 40 1105oxidized boundaries. 22-32 Interval: 4-5-5-9 CL Moist; 2% gravel, 4% sand, 94% fines, At 18': very fine to fine grained sand, sandy lean clay towards 20', oxidation, some red surrounded by black spots. 4-7-5-8. 1100 2.5Y 5/3 (light olive brown); 0% gravel, 5% sand, 95% fines, medium plasticity, very fine to fine with trace mediun grained sand, subrounded to subangular with more subangular, red oxidation, low effervescence. 4-4-4-4 Date Boring Started: 12/17/15 Remarks: 12/17/15 Date Boring Completed: DJZ Logged By: **Drilling Contractor:** Stevens Drilling Additional data may have been collected in the field which is not included on this log. SIMCO 7000 Drill Rig:

			rr Engine 4 West C				L	.OG OF	BORING SLAG)-1
BA	R	Bis	smarck, N lephone:	D 585	503				SHEET 2 OF 2	2
Projec Locati	t No.:4 on:Big inates	11251 Ston UTM	e Plant			work E:695191m	Surface Elevation:1120.3 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:32.3 ft	Top of Ca	sing Elev.: 1123.0 ft	
	Sample Type & Recovery	Sample No.	Blows/6in.	U S C S	Graphic Log		LITHOLOGIC DESCRIPTION	0.000	L OR PIEZOMETER CONSTRUCTION DETAIL	Elevation, feet
-25 - -	X	9	3-4-4-4.			LEAN CLAY (CL): 0% gravel Moist; At 26': medium plastic			PRO. CASING Diameter: 6" Type: Steel Interval: Surface + 3'	1095
30-		10	2-2-4-2.	CL		grained sand. 2.5Y 4/4 (olive brown): moist	ted; 0% gravel, 40% sand, 60% fines, very fine to fine ; 0% gravel, 5% sand, 95% fines, medium plasticity, very n grained sand, subrounded to subangular, medium		RISER CASING Diameter: 2" Type: PVC SCH 40 Interval: 0-25' bgs	1090
Micelatiperoject1941251001 BIG STONE;41251005 BIG STONE;GPJ BARRLIBRARY;GLB ENVIRO LOG BARR TEMPLATE;GDT Micelatipe and the property of the p						grained more subangular, ox End of boring 32.3 feet	idized red pockets, some larger areas, low effervescence.		GROUT Type: Neat cement Interval: with 5% bentonite slurry SEAL 3-17.5' bgs Type: Bentonite chips Interval: 0-20' bgs SANDPACK Type: AGSCO#1 Silica Interval: 50-80 20-32' bgs SCREEN Diameter: Slot 6 Type: 2" PVC SCH 40 Interval: 22-32'	
Date B Logged Logged Drilling	Date Boring Started: 12/17/15 Date Boring Completed: 12/17/15 Logged By: DJZ Drilling Contractor: Stevens Drilling					Drilling	Remarks: Additional data may have been collected in the field which is not included on this	log.		

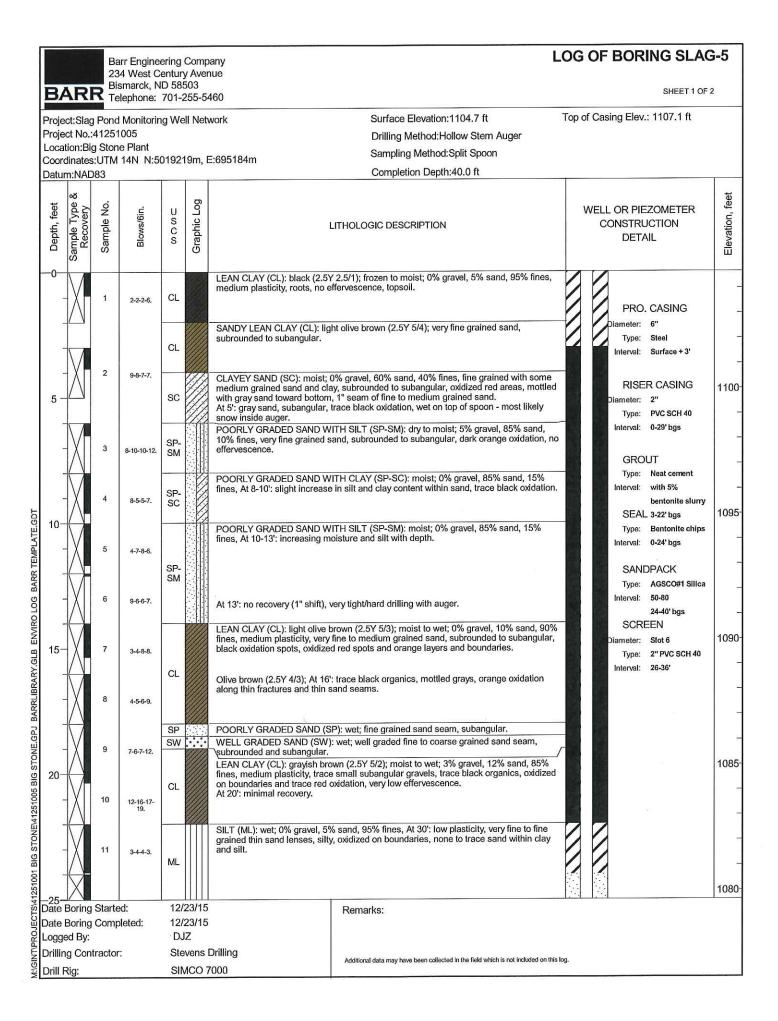
F			arr Engine			3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3. 3	L	OG OF	BORING SLAG	6-2
В	AR	Bi	smarck, N elephone:	D 58	503				SHEET 1 OF 1	1
Pro Loc Coc	ject No. ation:Bi	:41251 g Ston s:UTM	e Plant			work =:695093m	Surface Elevation:1108.3 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:21.0 ft	Top of Ca	sing Elev.: 1111.0 ft	
Depth, feet	Sample Type & Recovery	Sample No.	Blows/6in.	Uscs	Graphic Log	ЦТН	HOLOGIC DESCRIPTION		L OR PIEZOMETER ONSTRUCTION DETAIL	Elevation, feet
-0	-	1	4-4-3-4.			sand, subangular fill.	brown); frozen; very fine grained to medium grained	98	PRO. CASING	_
	1/1					10YR 2/2 (very dark brown); moist roots.	st; 0% gravel, 2% sand, 98% fines, At 1.5': plastic with		Diameter: 6" Type: Steel	_
5		2	3-4-5-4.			to medium grained sand, subroun	i; 0% gravel, 5% sand, 95% fines, nonplastic, very fine ided to subangular, medium grained more (light gray), oxidized with red spots, low		Interval: Surface + 3' RISER CASING Diameter: 2" Type: PVC SCH 40	1105-
		3	3-4-4-4.	CL		Moist; At 7' and 7.5': thin sand length	nses, black organics/ oxidation.		Interval: 0-13' bgs GROUT Type: Neat cement Interval: with 5%	1100-
MPLATE.GDT		4	5-6-6-5.	7,		Wet; 2% gravel, 8% sand, 90% fin throughout, oxidized on boundarie	nes, At 9': medium plasticity, thin sand lenses and fractures, red mottles with gray, soft.		bentonite slurry SEAL 3-6' bgs Type: Bentonite chips Interval: 0-8' bgs	_
BARR TE	-	5	4-6-6-8.			Wet; 0% gravel, 10% sand, 90% f	fines, At 12': sand lenses within.		SANDPACK Type: AGSCO#1 Silica	2
B ENVIRO LOG BARR TEMPLATE.GDT		6	3-5-4-5.	SC		grained sand, well graded, subrou effervesence.	2% gravel, 68% sand, 30% fines, fine to medium unded to subangular- mostly subangular, low to no		lnterval: 50-80 8-20,5' bgs SCREEN Diameter: Slot 6	1095-
BRARY.GLB		7	4-8-8-8.			Saturated; 5% gravel, 70% sand, 2 graded, subrounded to subangular	25% fines, very fine to coarse grained sand, very well ar with mostly subangular, trace subangular gravel.		Type: 2" PVC SCH 40 Interval: 10-20'	-
J BARRLI		8	7-5-7-7.	CL		subangular, increased silt, oxidize	d; nonplastic, trace medium grained sand, ed. 2% gravel, 60% sand, 38% fines, At 18': trace gravel.			-
STONE.GP.		G		sc			10% fines, At 19': 6" bottom is a black/ black and			1090-
M:GINTIPROJECTS/41251001 BIG STONE/41251005 BIG STONE.GPJ BARRLIBRARY.GLB M:GINTIPROJECTS/41251001 BIG STONE/41251005 BIG STONE.GPJ BARRLIBRARY.GLB G P G G G G G G G G G G G G G G G G G		6	7-7-6-8.			At 20': 6" subrounded to subangul 2.5Y 4/4 (olive brown); moist; 0% plasticity, fine to medium grained by by by trace black organics/ ox End of boring 21.0 feet	gravel, 10% sand, 90% fines, At 20.5': 6" medium sand, subrounded to subangular, soft, trace red		4)	-
Date Date Log	Date Boring Started: 12/18/18 Date Boring Completed: 12/18/18 Logged By: DJZ						Remarks:	·		
Drill Drill	Drilling Contractor: Stevens Drilling Drill Rig: SIMCO 7000						Additional data may have been collected in the field which is not included on this	log.		

			arr Engine					LOG O	WELL SLAG	2B	
B	4RI	D.	smarck, felephone:	10 50	FOO				SHEET 1 OF	1	
Proje Proje Loca	ect:Big ect No. ation:B	Stone :4125 ig Sto	e Station	SD			Surface Elevation:1108.5 ft Drilling Method:HSA Sampling Method:SS	Top of Ca	sing Elev.: 1110.5 ft		
			State Pla				Completion Depth:40.0 ft	_		1	
Depth, feet	Sample Type & Recovery	Sample No.	Blows/6in.	U S C S	Graphic Log	и	ITHOLOGIC DESCRIPTION		OR PIEZOMETER DNSTRUCTION DETAIL	Elevation, feet	
20-10-10-10-10-10-10-10-10-10-10-10-10-10			5-6-6-8. 6-8-11-10. 6-9-8-9. 3-3-5-8. 10-9-10-16. 5-8-8-11. 7-6-9-7.	CL CL SC		with very fine to medium grain s increasing with depth. LEAN CLAY (CL): dark gray (10 sand and gravel. 26.5': SILTY SAND (SM) seam, CLAYEY SAND (SC): w/ gravel gray (10YR 4/1).	wn (10YR 5/6); moist; 0% gravel, 5% sand, 95% fines, and and fine to coarse grain, subrounded gravel YR 4/1); moist; 15% gravel, 15% sand, 70% fines, with 1' thick. and large pockets of sand w/ silt, as logged by SET; dark YR 4/1); moist; 15% gravel, 15% sand, 70% fines, with		PRO. CASING Diameter: Type: Interval: RISER CASING Diameter: 2" Type: PVC Sch 40 Interval: GROUT Type: Neat Cement Interval: 0-29' bgs SEAL Type: Bentonite Interval: 29-31' bgs SANDPACK Type: Silica #50-80 Interval: 31-38' bgs SCREEN Diameter: Type: #6 Sch 40 Interval: 32.5-37.5' bgs	1105 1100 1095 1096 1085 1076	
40.			4-5-5-8.			End of well 40.0 feet					
Date Date Logo	Date Boring Started: 7/29/16 7:20 am Date Boring Completed: 7/29/16 9:30 am Logged By: JWJ						Remarks:	•			
Drilli Drill	Illing Contractor: SDE Il Rig: Truck						Additional data may have been collected in the field which is not included on this log.				

(F			arr Engine				nego disc	OG O	BORING SLAG	3- 3
BA	\R	D:	34 West C smarck, N elephone:	ID FO	200				SHEET 1 OF	1
Project	ct No. ion:Bi inates	:41251 g Ston s:UTM	e Plant			work E:695252m	Surface Elevation:1108.3 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:25.0 ft	Top of Ca	sing Elev.: 1111.5 ft	
985,000	Sample Type & Recovery	Sample No.	Blows/6in.	Uscs	Graphic Log	LITHO	OLOGIC DESCRIPTION	1.200000	L OR PIEZOMETER CONSTRUCTION DETAIL	Elevation, feet
-0-		1	13-12-12- 10.	CL		LEAN CLAY WITH SAND AND GF (2.5Y 3/3); frozen; 10% gravel, 10% subrounded to subangular, mostly	RAVEL (CL): black (2.5Y 2.5/1) to dark olive brown % sand, 80% fines, low plasticity, sand and gravels / subangular, low effervescence.		PRO. CASING Diameter: 6"	
5 —		2	5-3-3-4.			LEAN CLAY (CL): olive brown (2.5 sand, 90% fines, fine to medium groxidation, low effervescence.	5Y 4/4); moist; medium plasticity, 2% gravel, 8% grained sand, subrounded to subangular, trace red		Type: Steel Interval: Surface + 3' RISER CASING Diameter: 2"	1105
-		3	5-9-7-9.	CL			d, 90% fines, At 8': sandy clay, fine grained sand,		Type: PVC SCH 40 Interval: 0-13' bgs GROUT	1100
10- 10-		4	5-8-10-13.				d gray. ines, At 9': LEAN CLAY WITH SAND sand and small gravel, trace black organics.		Type: Neat cement Interval: with 5% bentonite slurry SEAL 3-6' bgs	
WIRO LOG BARR TEMPLATE.GDT G1 1	\bigvee	5	10-10-11- 10.	SM		Wet; 2% gravel, 13% sand, 85% fil SILTY SAND (SM): wet; At 13': 4" : subangular, low to no effervescend	SILTY SAND, very fine grained, subrounded to		Type: Bentonite chips Interval: 0-8' bgs SANDPACK	1095
VIRO LOG BAR	$\bigvee_{i=1}^{N}$	7	5-5-7-10. 3-5-9-11.			LEAN CLAY (CL): olive brown (2.5 0% gravel, 5% sand, 95% fines, At	5Y 4/4).		Type: AGSCO#1 Silica Interval: 50-80 8-25' bgs SCREEN	2
		8	5-6-6-11.	CL			. 95% fines, At 18': Two thin sand seams, very fine to subangular, low to no effervescence.		Diameter: Slot 6 Type: 2" PVC SCH 40 Interval: 10-20'	1090
EGPJ BARRLI		9	5-7-11-13.			9	95% fines, At 20': Two thin sand lenses, very fine to			2
005 BIG STON		10	6-8-9-14.				5Y 4/4); wet; 0% gravel, 10% sand, 90% fines, low			1085
Mikolimprojects/47251001 Big stone/47251005 Big stone/get Barriberary.clb Ep. 1		11	6-7-7-11.	CL			ined sand, subrounded to subangular, more seed red, some thin sand lenses oxidized on ow to no effervescence.			=
Date E	Boring			12/	21/15 22/15		Remarks:			
Logge Drilling Drill R	g Con	tractor	:				udditional data may have been collected in the field which is not included on thi	s log.		



Ę	um		arr Engine 34 West C					LOG O	F BORING SLAG	3-4
B	٩R	D:	smarck, N elephone:	ID E0	E00				SHEET 2 OF 2	2
Proje Proje Loca Coor	ect:Sla ect No. tion:B	g Pond :4125 g Stor s:UTM	d Monitorir 1005 ne Plant	ng We	ell Net	V	Surface Elevation:1110.2 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:37.0 ft	Top of C	casing Elev.: 1113.2 ft	
Depth, feet	Sample Type & Recovery	Sample No.	Blows/6in.	USCS	Graphic Log		LITHOLOGIC DESCRIPTION	WE	ELL OR PIEZOMETER CONSTRUCTION DETAIL	Elevation, feet
25- - -	X	13	4-4-4-6. 4-4-4-6.	CL		sand, 95% fines, high plastic Wet to saturated; At 26.25': 2	grayish brown (2.5Y 3/2); moist to wet; 0% gravel, 5% ity, trace silt, lean, oxidized, none to low effervescence?" wet to saturated sandy clay seam. I, 5% sand, 95% fines, high plasticity, several thin very subrounded and subangular, trace black organics, mot	/I: H	PRO. CASING Diameter: 6" Type: Steel Interval: Surface + 3'	108
30-		14	5-9-10-16. 18-13-13- 19.	CL		* *	08% fines, At 30': fine grained sand lense. //ITH CLAY (SP-SC): saturated; 2% gravel, 88% sand,		RISER CASING Diameter: 2" Type: PVC SCH 40 Interval: 0-29' bgs	108
35-		16	2-1-1-1.	SP- SC		10% fines, very fine to fine gr	rained sand, subrounded to subangular.		GROUT Type: Neat cement Interval: with 5% bentonite slurry SEAL 3-22' bgs Type: Bentonite chips Interval: 0-24' bgs	107
35- - - - 40-						End of boring 37.0 feet			SANDPACK Type: AGSCO#1 Silica Interval: 50-80 24-37' bgs SCREEN Diameter: Slot 6 Type: 2" PVC SCH 40 Interval: 26-36'	
40- 45- Date Loggo Drillir										
Date Logge	Boring ed By:		oleted:	12/ D.		i	Remarks:			1
Drill I		itracto			wens MCO	Drilling 7000	Additional data may have been collected in the field which is not included o	n this log.		



			arr Engine				ı	.OG OF	BORING SLAG	3-5
В	AR	D:	4 West C smarck, N elephone:	D EO	500				SHEET 2 OF 2	2
Proj Proj Loca Coo	ect:Sla ect No. ation:B rdinate	g Pond :41251 ig Ston s:UTM	l Monitorir 1005 e Plant	ng We	II Net		Surface Elevation:1104.7 ft Drilling Method:Hollow Stem Auger Sampling Method:Split Spoon Completion Depth:40.0 ft	Top of Ca	sing Elev.: 1107.1 ft	
Datu	JM:NAI	283					Сопрецоп Берип.40.0 п			
Depth, feet	Sample Type & Recovery	Sample No.	Blows/6in.	U S C S	Graphic Log		ITHOLOGIC DESCRIPTION	1,200,000	L OR PIEZOMETER CONSTRUCTION DETAIL	Elevation, feet
-25	X	12	5-4-4-6.			SILT (ML): wet; 0% gravel, 5% grained thin sand lenses, silty, and silt. (continued)	sand, 95% fines, At 30': low plasticity, very fine to fine oxidized on boundaries, none to trace sand within clay			_
		13	6-9-8-9.			Wet; 0% gravel, 5% sand, 95% boundaries, fractured, very we Wet; 0% gravel, 5% sand, 95%			PRO. CASING	Ē
				ML			% gravel, 5% sand, 95% fines, At 28': low plasticity.		Type: Steel Interval: Surface + 3'	-
		14	4-5-6-5.						RISER CASING	1075
30		15	4-5-5-12.	CL		medium plasticity, fine to coars subangular coarse grains, thin boundaries.	(2.5y 4/3); wet; 0% gravel, 10% sand, 90% fines, se grained sand, subrounded and subangular, more a sand lenses with orange and black oxidation on		Diameter: 2" Type: PVC SCH 40 Interval: 0-29' bgs	-
		16		J.		mottled clay with gray, very lov	nded to subangular, red oxidation spots, 6" area of w effervescence, 1/2 inch saturated clayey sand.		GROUT Type: Neat cement	-
		10	4-3-12-8.	SM		Coarse grained sand, mostly fi LEAN CLAY (CL): dark gray (2	(2.5y 4/1); wet; 5% gravel, 20% sand, 75% fines, fine to ne grained, well graded subrounded to subangular. 2.5y 4/1); moist to wet; fine to coarse grained sand,	1	Interval: with 5%	-
7E.GDT	subangular, trace subangular gravels, no oxidation, low effervescence. At 35': no recovery.								SEAL 3-22' bgs Type: Bentonite chips	1070-
ENVIRO LOG BARR TEMPLATE.GDT		18	15-16-21- 25.	CL		Moist to wet; 5% gravel, 10% s trace subangular gravels, no o	sand, 85% fines, fine to coarse grained sand, subangular xidation, low effervescence.		Interval: 0-24' bgs SANDPACK Type: AGSCO#1 Silica Interval: 50-80	-
NVIRO LOC		19	4-5-5-8.			Moist to wet; 5% gravel, 5% sa	and, 90% fines, medium to high plasticity.		24-40' bgs SCREEN Diameter: Slot 6	1065
	_/_N					End of boring 40.0 feet			Type: 2" PVC SCH 40	
M:GINTPROJECTS/4125/1001 BIG STONE/4125/1005 BIG STONE.GPJ BARRLIBRARY.GLB M: Q										
45 45										
3IG STONE/412	-									
251001 E	-									
Date Date Logg	Boring Boring Boring Bed By:	g Comp	oleted:	12/. DJ		5	Remarks:		L	1
Drilli Drill	ing Cor Rig:	ntracto	r:			Drilling 7000	Additional data may have been collected in the field which is not included on this	log.		

			arr Engine 34 West C					LOG	OF WELL SLA	G 6
В	AR	D:	smarck, N elephone:	ID FO	FAA				SHEET 1 OF	÷ 1
Proje Loca Coor	ect No. ation:B dinate	:4125 ig Stor s:N 55	Station 1005 ne City, SE 4,058.1 ft State Plar	E 2,		10.7 ft	Surface Elevation:1108.5 ft Drilling Method:HSA Sampling Method:SS Completion Depth:34.5 ft	Top of Ca	sing Elev.: 1111.5 ft	
Depth, feet	Sample Type & Recovery	Sample No.	Blows/6in.	USCS	Graphic Log		LITHOLOGIC DESCRIPTION		L OR PIEZOMETER CONSTRUCTION DETAIL	Elevation, feet
-0-	X		3-2-2-2.	CL		FILL - SANDY CLAY (CL): b with slag.	lack (10YR 2/1); moist; 5% gravel, 15% sand, 80% fines,			
8	X		5-4-4-4.			LEAN CLAY (CL): yellowish trace subrounded gravel.	brown (10YR 5/4); moist; 5% gravel, 0% sand, 95% fines,		PRO. CASING	110
5 -	X		2-4-5-5.	CL		3.11			Type: Steel Interval:	
	X		4-5-7-5.						RISER CASING	
10-	X		6-7-9-9.	SM		-25% fines.	owish brown (10YR 44/4); moist; 5% gravel, 70% sand, brown (10YR 6/2); moist; 10% gravel, 5% sand, 85% fines		Diameter: 2" Type: PVC Sch 40 Interval:	110
			6-7-7-7.	CL		with coarse grain, subangula	ar gravel, rusty yellowish red oxidized seams.		GROUT	
6-	X		5-7-6-7.	-					Type: Neat Cement Interval: 0-17,5' bgs	109
15-			3-3-5-5.	_SM_		SILTY SAND (SM): brown (1 fine grain.	0YR 6/2); moist; 0% gravel, 80% sand, 20% fines, very	7	SEAL	
2	X		5-5-7-10.			LEAN CLAY (CL): yellowish	brown (10YR 5/6); moist; 10% gravel, 5% sand, 85% fines es sandy lean clay with depth.		Type: Bentonite Interval: 17.5-20.5' bgs	
20-	X		12-7-10-14.					88	SANDPACK Type: Silica #50-80	109
-	X		6-6-6-9.						Interval: 20.5-34.5' bgs	
			7-8-7-7.						SCREEN Diameter:	108
25-			5-6-6-11.	CL					Type: #6 Sch 40 Interval: 24,2-34,2' bgs	5
2			8-11-10-15.							
30-	X		7-11-13-25.			clay, 0% gravel, 80% sand, 2				108
-	X		9-5-10-11.			30': CLAYEY SAND (SC) wit 40% sand, 30% fines, 1' thic	h gravel, yellowish brown (10YR 5/6), wet, 30% gravel, k.			
-										107
35-	X		50/0			End of well 34.5 feet				
-										
	Boring					2:20 pm	Remarks:			
Logge	Boring ed By:			JW						
Drillin Drill F	g Con Rig:	ractor		SDI Tru		7	Additional data may have been collected in the field which is not included on this	log.		

		Ra	arr Engine	erina	Com	pany		LOG	OF WELL SLA	G 7
BA	١R	23	34 West C smarck, N elephone:	entur	y Ave	nue			SHEET 1 OF	:1
			Station				Surface Elevation:1111.9 ft	Top of Cas	sing Elev.: 1114.4 ft	
Proje	ct No.	:41251	1005	28			Drilling Method:HSA			
			ie City, SI 4,435.7 ft		865.4	.83 4 ft	Sampling Method:SS			
100000000000000000000000000000000000000			State Plar			00,111	Completion Depth:46.0 ft			
Depth, feet	Sample Type & Recovery	Sample No.	Blows/6in.	U S C S	Graphic Log		LITHOLOGIC DESCRIPTION		L OR PIEZOMETER CONSTRUCTION DETAIL	Elevation, feet
-0-	M		20.0000000	CL): black to olive brown (2.5Y 2.5/1); moist; 0% gravel, 5%			
5 -			3-3-4-5. 7-8-7-7. 3-2-5-6.			rusty colored seams, trace ve	m (2.5U 4/4); moist; 0% gravel, 5% sand, 95% fines, with ery fine grain to coarse grain sand throughout, trace ut, some <1" lenses of sand and silt mixture (SM), lean		PRO. CASING Diameter: 6" Type: Steel	1110
-			6-6-9-11.						Interval:	110
10-			7-8-7-9.						RISER CASING	
10-	X		6-7-7-9.			10': CLAYEY SAND (SC), oli of sand/silt mixture and sand 40% fines, 1' thick.	ive brown (2.5Y 4/4), moist to saturated, alternating layers I/clay mixture about 3" thick each, 0% gravel, 60% sand,		Type: PVC Sch 40 Interval:	1100
-	X		6-7-7-9.							
15-	X		4-5-5-6.	CL		15': CLAYEY SAND (SC), oli 1.5' thick,	ive brown (2.5Y 4/4), wet, 0% gravel, 60% sand, 40% fines	з,	GROUT Type: Neat Cement	
-	X		5-4-5-5.	OL.		1.5 thick.			Interval: 0-30,8' bgs	1098
5 E	X		7-6-5-5.						SEAL	
20-	X		9-5-6-8.						Type: Bentonite Interval: 30.8-33' bgs	1090
-	X		5-5-5-7.						SANDPACK	1000
25-	X		5-4-4-5.						Type: Silica #50-80 Interval: 33-47' bgs	
9 <u>=</u>	X		5-4-4-4.						SCREEN	108
30-	X		4-4-4-4.			29': SILTY SAND (SM), dark 70% sand, 30% fines, 1.2" th	grayish brown (2.5Y 4/2), moist, very fine grain, 0% grave ick.		Diameter: Type: #6 Sch 40	
- - -	X		5-4-6-8.	\SC/		very fine grain.	rown (2.5U 4/4); wet; 0% gravel, 70% sand, 30% fines, live brown (2.5U 4/3); wet to saturated; 10% gravel, 10%	777	Interval: 36-46' bgs	1080
35-			5-4-3-3.			sand, 80% fines, very fine gra	ain, becomes sandier with depth. ve brown (2.5Y 4/3), very fine grain to medium grain, 0%			
-	X		7-8-10-11.	CL		gravel, 80% sand, 20% fines	, 9.6" thick.			107
40-		ŝ	6-9-17-23.			40': CLAYEY SAND (SC), oli 0% gravel, 60% sand, 40% fi	ive brown (2.5Y 4/3), wet, very fine grain to medium grain, ine 2' thick			22222
-						C/o gratori co /o carret re/o				1070
45-			10-13-19- 18. 8-8-14-21.	ML			4/3); wet; 0% gravel, 0% sand, 100% fines, with trace ver	у		
-						_fine grain sand. End of well 46.0 feet				
-50- Date	J Borinc	Starte	 ed:	7/2	0/16	2:23 pm	Remarks:			
Date	Boring	Comp	oleted:	7/2	1/16	:50				
	ed By:		,,	JW SD						
	_	itractor	9	Tru			Additional data may have been collected in the field which is not included on the	is log.		
Drill F	vig.			111	ION		ļ			

		_					4		
TO THE		234	rr Engine 4 West C	entur	y Ave			LOG OF WELL SLAG	š 8
BA	RF	Bis	marck, N lephone:	ND 58	503			SHEET 1 OF 1	1
Project Location Coordin	No.:4 n:Big nates:	1251 Stor N 55	Station 1005 ne City, S 3,999.3 f State Pla	t E 2			Surface Elevation:1124.3 ft Drilling Method:HSA Sampling Method:SS Completion Depth:55.0 ft	Top of Casing Elev.: 1127.3 ft Unique Well No.: BONUS WELL	5
Depth, feet		Sample No.	Blows/6in.	Uscs	Graphic Log		LITHOLOGIC DESCRIPTION		Elevation, feet
5 - 10 - 10 - 1			6-4-4-9. 7-6-9-12.			LEAN CLAY (CL): dark yellowi	ish brown (10YR 4/4); moist; 5% gravel, 15% sand, 80%	fines, with sand, oxidization throughout.	11120
15-2			15-26-18- 22. 14-13-19- 23.	CL		17": 1" gravel layer with cobble	es, subangular. dium grained; dark yellowish brown (10YR 4/4); wet; 20%	6 gravel, 50% sand, 30% fines, with gravel.	1105
25			8-15-13- 11. 11-11-18- 13.	SM		23': WELL GRADED SAND (S	SW) with gravel, dark yellowish brown (10YR 4/4), wet, 15	5% gravel, 85% sand, 5% fines, 3' thick.	1100
30 30 35 35 35 35 35 35 35 35 35 35 35 35 35			8-8-11-13. 13-9-10- 15. 5-6-11-12.	CL		LEAN CLAY (CL): dark yellowi 29.5': 1" fine to medium grain	ish brown (10YR 4/4); moist; 0% gravel, 10% sand, 90% sand lense.	fines, with sand, oxidization throughout.	1095
355 40 445 1000 Bild Strongs Brakktiskakry 15th Bra			7-9-11-16. 4-5-8-11. 6-7-9-11. 4-4-4-7. 7-5-6-7.	CL		LEAN CLAY (CL): very dark gi	ray (10YR 3/1); moist; 0% gravel, 10% sand, 90% fines.		1085
251005 BIG STONE GPJ BAP			11-15-18- 21.	SM CL		LEAN CLAY (CL): very dark g End of well 55.0 feet	wish brown (10YR 4/4); moist; 0% gravel, 60% sand, 40% ray (10YR 3/1); moist; 0% gravel, 10% sand, 90% fines.	6 fines.	1070
Date Bo	oring d By: Conti	Com	pleted:	8/1 NJ: SD	/16 3 S2 E	1:30 am :30 pm	Remarks: Boring advancd next to existing SLAG 8 well Additional data may have been collected in the field which is not included		
Drill Rig	g:			Tru	ıck				

Barr Engineeri			LOG OF WELL SLAG	9
BARR 234 West Cen Bismarck, ND Telephone: 70	EOEOO		SHEET 1 OF 1	
Project:Big Stone Station		Surface Elevation:1119.4 ft	Top of Casing Elev.: 1122.4 ft	
Project No.:41251005		Drilling Method:HSA		
Location:Big Stone City, SD Coordinates:N 554,758.4 ft E	- 2 866 219 4 ft	Sampling Method:SS		
Datum:SD North State Plane		Completion Depth:48.0 ft		
Depth, feet Sample Type & Recovery Sample No. Blows/6in.	Graphic Log	LITHOLOGIC DESCRIPTION	WELL OR PIEZOMETER CONSTRUCTION DETAIL	Elevation, feet
10- 15- 20- 25- 30- 30- 30- 35- 40- 6-12-12-13.	SILTY SAND (SM): very fine to gravel, 75% sand, 25% fines, 39': POORLY GRADED SAN fine to fine grain, 0% gravel, 5	to fine grained; yellowish brown (10YR 5/4); wet; 0% gravel content increases with depth. D WITH SILT (SP), yellowish brown (10YR 5/4), wet, very 90% sand, 10% fines, 1' thick. wn (10YR 5/4), wet, 0% gravel, 10% sand, 90% fines, 2'	PRO. CASING Diameter: 6" Type: Steel Interval: RISER CASING Diameter: 2" Type: PVC Sch 40 Interval: GROUT Type: Neat Cement Interval: 0-31.5' bgs SEAL Type: Bentonite Interval: 31.5-35' bgs SANDPACK Type: Silica #50-80 Interval: 35-50' bgs SCREEN Diameter: Type: #6 Sch 40 Interval: 38-48' bgs	1110- 1110- 1100- 11090- 1095- 1085- 1080-
	End of well 48.0 feet			
Date Boring Started:	7/27/16 2:45 pm	Remarks:	trade Erad	
Date Boring Completed:	7/28/16 5:15 pm	0 data a sat 40 T a		
	JWJ			
[- · · · · · · · · · · · · · · · · · ·	SDE	Additional data may have been collected in the field which is not included on this	log.	
Drill Rig:	Truck			

LOG OF WELL SLAG 9 DEEP Barr Engineering Company 234 West Century Avenue Bismarck, ND 58503 BARR Telephone: 701-255-5460 SHEET 1 OF 1 Top of Casing Elev.: 1121.9 ft Surface Elevation:1119.4 ft Project:Big Stone Station Project No.:41251005 Drilling Method: HSA Location:Big Stone City, SD Sampling Method:SS Coordinates:N 554,758.4 ft E 2,866,219.4 ft Datum:SD North State Plane NAD83 Completion Depth:68.0 ft feet Sample Type WELL OR PIEZOMETER feet Recovery USCS Blows/6in Elevation, Graphic 1 Sample ! CONSTRUCTION Depth, 1 LITHOLOGIC DESCRIPTION **DETAIL** LEAN CLAY (CL): dark yellowish brown (10YR 4/5); moist; 10% gravel, 10% sand, 80% 4-3-3-4. fines, with sand and coarse grain, subangular gravel throughout, gravel content increasing with depth. 15-13-12-10. PRO. CASING 1115 3-5-4-5. Type: Interval: 1110-7-6-10-11. 10 6-5-8-10. RISER CASING 7-9-10-10. 1105 Diameter: 2" CL 7-8-7-8. Type: PVC Sch 40 Interval: 3-3-5-7. 1100 5-7-7-10. 20 GROUT 6-8-8-10. Type: Neat Cement 9-14-14-14 Interval: 0-58,5' bgs 1095 25 6-7-9-12. SEAL Type: Bentonite 1090 CLAYEY SAND (SC): very fine to fine grained; dark yellowish brown (10YR 4/5); wet; 5% gravel, 75% sand, 20% fines, with coarse grain, subangular gravel. 12-13-18-24. Interval: 58,5-61' bgs 14-18-20-22, SC SANDPACK 22-36-16-1085 Type: Silica #50-80 LEAN CLAY (CL): dark yellowish brown (10YR 4/5); moist; 10% gravel, 20% sand, 70% CL 15-10-24-32. ENVIRO LOG BARR TEMPLATE, GDT fines, with sand and gravel. Interval: 61-68' bgs 5-6-8-13. SILTY SAND (SM): wet; 0% gravel, 80% sand, 20% fines, very fine grain to medium grain, some fine grain gravel. 1080 **SCREEN** 7-6-12-13. #6 Sch 40 26-24-10-8 SM Interval: 63-68' bgs 1075 8-9-9-10 9-11-9-15. 47.8': CLAYEY SAND (SC), dark yellowish brown (10YR 4/6), wet, very fine grain, some gravel, 10% gravel, 70% sand, 20% fines, 2.4" thick. 1070-9-11-9-15. 49: SILTY GRAVEL (GM) grades to CLAYEY GRAVEL (GC), dark yellowish brown (10YR 4/6), wet, 50% gravel, 30% sand, 20% fines, 3' thick. BARRLIBRARY.GLB 9-8-14-16. LEAN CLAY (CL): very dark gray (2.5Y 3/1); moist; 5% gravel, 5% sand, 95% fines, 11-13-14-1065 some coarse grain, subangular gravel, fine to medium grain sand lense. 12-14-11-16. 8-8-7-10. 1060 6-4-7-9. CL ECTS\41251005 BIG STONE.GPJ 60 9-10-10-14 1055 65 9-11-26-37. ML \SILT (ML): gray (2.5Y 5/1); moist. End of well 68.0 feet Date Boring Started: 7/22/16 8:50 am Remarks: Date Boring Completed: 7/22/16 Logged By: JWJ SDE **Drilling Contractor:** Additional data may have been collected in the field which is not included on this log. Drill Rig: Truck

Appendix C

Geotechnical Laboratory Results

Hydraulic Conductivity Test Data ASTM D2434

Pro	ject:			Big Stone			Da	ate: _	8/29/2016
Clie	ent:		Bar	r Engineering	Company		Job N	10.:	10490
Bor	ing No.:	Slag 9 Deep							
San	nple No.:								
	oth (ft):	38-40							
Loc	ation:								
San	nple Type:	ЗТ							
		Sand with silt (SP-SM)							
	Type: rberg Limits			-				\dashv	
				1				+	
느								\dashv	
<u>Р</u> Р								\dashv	
	neability Test	Intact			+			\dashv	
								寸	2
lition T	orosity:								
Sond T	aturation %: orosity: It. (in): via. (in): ory Density (pcf): Vater Content:	3.97						\neg	
est (ia (in):	2.88					48	7	
ore T	ny Density (ncf):	111.7						1	
Befe	Vater Content:	17.7%						\dashv	
	est Type:	Constant						\top	
100	lax Head (cm):	28.2							
C	onfining press. Effective-psi):	None							
T	rial No.:	6-10							
V	/ater Temp °C:	21.0							
9	6 Compaction								
	% Saturation (After Test)								
				Coefficient of	of Permeability	/	<u> </u>	_	
Κ@	20 °C (cm/sec)	1.5 x 10 ⁻⁴						4	
Κ@) 20 °C (ft/min)	3.0 x 10 ⁻⁴						\perp	
	Notes:						*		
	91	530 James Ave South		FOIL	EERING	Bloom	nington, MN 55431		

Hydraulic Conductivity Test Data ASTM D5084

Project: Big Stone Date: 9/7/20 Client: Barr Engineering Company Job No.: 1049													
Client:													
Boring No.:													
Sample No.:					SLA	G 2B							
Depth (ft):	16	3-18	47	7-49	28	3-30	32-34						
Sample Orientation:	ple Orientation: Vertical Horizontal Vertical Horizontal Vertical												
Sample Type:	3	зт		3T	3	ВТ	3T						
	Sandy Lean Clay W/ a little gravel (CL) Sandy Lean Clay Sandy Lean Clay W/ gravel and I pockets of sand (SC)												
Soil Type:													
Atterberg Limits													
<u>LL</u>													
PL													
PI		V-1//4		1.00	Potroi	Letest	Intent						
Permeability Test	Intact	Intact	Intact	Intact	Intact	Intact	Intact						
Saturation %: Porosity: Ht. (in): Dia. (in): Dry Density (pcf): Water Content:							0.050						
등 Porosity:	0.339	0.358	0.331	0,346			0.352						
び Ht. (in): ば	2.95	2.13	2.91	2.07	2.86	1.59	3.03						
Dia. (in):	2.87	1.94	2,88	1.94	2.90	1.94	2.88						
Dry Density (pcf):	110.6	107.4	111.9	109.4	121.3	116.7	108.4						
^m Water Content:	18.3%	19.0%	17.2%	18.1%	10.7%	13.9%	18.3%						
Test Type:	Falling Head	Falling Head	Falling Head	Falling Head	Falling Head	Falling Head	Falling Head						
Max Head (ft.):	5.0	5.0	5.0	5.0	5.0	5.0	5.0						
Confining press. (Effective-psi):	2.0	2.0	2.0	2.0	2.0	2.0	2.0						
Trial No.:	7-11	9-13	7-11	10-14	7-11	10-14	7-11						
Water Temp °C:	22.0	22.0	22.0	22.0	22.0	22.0	22.0						
% Compaction													
% Saturation (After Test)	(After Test) 97.8% 95.2% 95.6% 95.2% 95.1%												
		100	Coefficient of	-									
K @ 20 °C (cm/sec)		3.6 x 10 ⁻⁸	2.8 x 10 ⁻⁸	4.8 x 10 ⁻⁸	1.0 x 10 ⁻⁶	1.2 x 10 ⁻⁶	6.6 x 10 ⁻⁷						
K @ 20 °C (ft/min)	1.3 x 10 ⁻⁷	7.1 x 10 ⁻⁸	5.5 x 10 ⁻⁸	9.5 x 10 ⁻⁸	2.0 x 10 ⁻⁶	2.4 x 10 ⁻⁶	1.3 x 10 ⁻⁶						
Notes:													
9!	530 James Ave South		FOIL NGINEI	ERING , INC.	Bloomi	ngton, MN 55431							

Hydraulic Conductivity Test Data ASTM D5084

Pro	ject:		04/240	Big Stone	Grades and the second 		Date:	9/7/2016
Clie	ent:		Barr E	Engineering Cor	mpany		Job No.: _	10490
Bori	ng No.:	SLAG 9 DEEP						
San	nple No.:							
Dep	th (ft):	64-66						
San	nple Orientation:	Vertical						
San	nple Type:	3Т						
		Sandy Lean Clay w/ a little gravel (CL)						
	Type:							
	rberg Limits							
<u>L</u>				¥				
<u>P</u>			=					
P								
	neability Test							
tions	aturation %.	0.405						
ondi	aturation %: orosity: t. (in): ia. (in): ry Density (pcf): Vater Content:	0.405						
est C	t. (in):	2.56						
re Te	la. (in):	2.88						
Befo	ry Density (pcr):	99.5						
		25.0%						
	est Type:	Falling Head						
C	lax Head (ft.): onfining press. Effective-psi):	2.0						
-	rial No.:	7-11						
-	/ater Temp °C:	22.0						
	6 Compaction							
	% Saturation (After Test)	99.6%						
			(Coefficient of F	Permeability			
K @	20 °C (cm/sec)	3.7 x 10 ⁻⁸						
K @) 20 °C (ft/min)	7.3 x 10 ⁻⁸						
	Notes:						ř.	
	98	530 James Ave South		FOIL	RING	Bloomi	ngton, MN 55431	

					Grair	Siz€) Di	istribut	tion As	ST	ΜI	D4	22				_	Jo	ob N	lo. :	10	049	0
	Project: B															1			st Da		_	24/41	_
Repor	ted To: B	arr Enginee	ering Com	pany													Re	por	rt Da	ate:	8/:	/26/16	3
	Location /	/ Boring No.	Sam	nple No.	Depth (ft)	Sample Type						S	oil Cla	assification	on								
* []	Н8			16-18	3T				Sa	ndy L	.ean (Clay	w/a littl	le grav	/el ((IL)						
•	SB H	I6 Deep			47-49	3T						Sano	dy Le	an Clay	(CL)		-02						
♦ [Sl	lag 6			32-34	3T	<u> </u>			C	layey	Sand	dw/a	a trace o	f grave	el (S	C)						
		Grave			T	Mod	San	nd	Fina			_		Н	ydrom	neter Fine		alysi	S			\neg	
100	Co	oarse 3	Fine 74 3/8	e	Coarse #10	Medi #2	1um 30	#40	Fine #100	#2)) 	_	_			Fine	<u>s</u>	_	_	_	_	\dashv	
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	prosity				-	#4 93.		98.7	97.7	1													
Organi	ic Content				#	10 88.	.7	94.3	97.5														
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(* = a	ssumed)											-200				_							
							OIL	, ,	TNIC														

	Grain Size Distribution ASTM D422 Job No.: 10490										
	Project: Big St	one						Test Date: 8/24/416			
Reno	rted To: Barr I	Ingineeri	ing Company					Report Date: 8/26/16			
Nepoi					Sample			Troport E s.c.			
	Location / Bor	ing No.	Sample No.	Depth (ft)	Туре	<u> </u>	Soil Classification				
Spec 1	H8			16-18	3T	Saı	ndy Lean Clay w/a little grav	rel (CL)			
Spec 2	SB H6 De	ер		47-49	3T		Sandy Lean Clay (CL)				
Spec 3	Slag 6	el (SC)									
						Sieve Data					
Specimen 1 Specimen 2 Specimen 3											
	Sieve		% Passing		Sieve		Sieve	% Passing			
	2"				2"		2"				
	1.5"				1.5"		1.5" 1"				
	1"		100.0		1"		3/4"				
	3/4"		100.0	+-	3/4"	100.0	3/4"	100.0			
	3/8" #4	-	95.4 93.4	+	#4	98.7	#4	97.7			
	# 4 #10	+	88.7		#10	94.3	#10	97.5			
	#20	+	85.3	1	#20	90.0	#20	96.7			
	#40	+	81.2	_	#40	85.1	#40	95.9			
	#100		69.9		#100	71.8	#100	83.7			
	#200		59.3	1	#200	61.8	#200	49.0			
					H	ydrometer Data					
	Spe	cimen '	1			Specimen 2		Specimen 3			
Diar	neter (mm)		% Passing	Ι	Diamet		Diameter	% Passing			
	0.031		45.4		0.031	48.0	0.033	35.3			
	0.020		38.1		0.020		0.021	31.7			
	0.012		32.5		0.012		0.012	28.7 25.6			
-	0.009	+	27.8 23.8		0.009		0.009 0.006	22.9			
	0.006	+	17.7	_	0.003		0.003	17.3			
	0.003	-	14.3		0.001	15.3	0.001	13.6			
	0.001				15345-503	Remarks					
	Spe	cimen 1	1			Specimen 2	5	Specimen 3			
	9530 James Ave South OIL NGINEERING ESTING, INC. Bloomington, MN 55431										

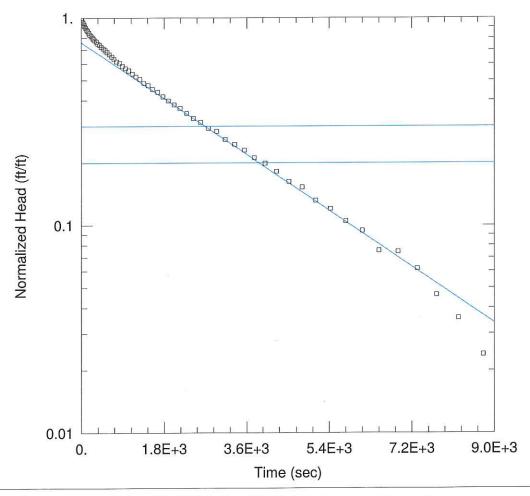
Grain Size Distribution ASTM D422 Job No.: 10490																							
F	Project: Big Stone											Test Date:					/24/16						
	oorted To: Barr Engineering Company										I	Rep	ort [Date:	8/	/26/16							
Sample Location / Boring No. Sample No. Depth (ft) Type Soil Classification																							
ъ. Г			Sam	pie ivo.	100 A 100 A																		
*	Slag	9 Deep	+		64-66	3T	+	Sandy Lean Clay w/gravel (C									-L)	-					
♦			+-		+	-	+																
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Appendix D

Slug Test Results



SLAG 1 FALLING HEAD SLUG TEST (SLUG-IN)

Data Set: \...\Slag 1 Slug In.aqt

Date: 09/23/16 Time: 11:57:15

PROJECT INFORMATION

Company: <u>Barr Engineering Co.</u> Client: OtterTail Power Company

Project: 41251005 Location: Big Stone, SD Test Well: Slag 2

Test Date: Aug 12, 2016

AQUIFER DATA

Saturated Thickness: 12. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 1)

Initial Displacement: 1.281 ft

Total Well Penetration Depth: 12. ft

Casing Radius: 0.083 ft

Static Water Column Height: 17.1 ft

Screen Length: 10. ft Well Radius: 0.344 ft

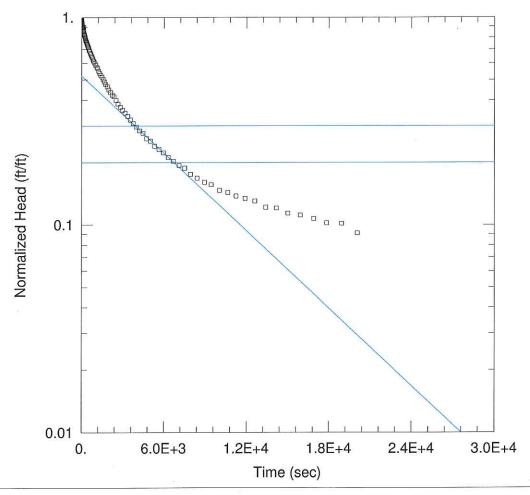
SOLUTION

Aquifer Model: Confined

K = 9.642E-6 cm/sec

Solution Method: Bouwer-Rice

y0 = 0.9758 ft



SLAG 1 RISING HEAD SLUG TEST (SLUG-OUT)

Data Set: \...\Slag 1 Slug Out.aqt

Date: 09/23/16

Time: 11:58:51

PROJECT INFORMATION

Company: <u>Barr Engineering Co.</u> Client: OtterTail Power Company

Project: 41251005

Location: Big Stone, SD

Test Well: Slag 2

Test Date: Aug 12, 2016

AQUIFER DATA

Saturated Thickness: 12. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 1)

Initial Displacement: 1.319 ft

Total Well Penetration Depth: 12. ft

Casing Radius: 0.083 ft

Static Water Column Height: 17.1 ft

Screen Length: 10. ft Well Radius: 0.344 ft

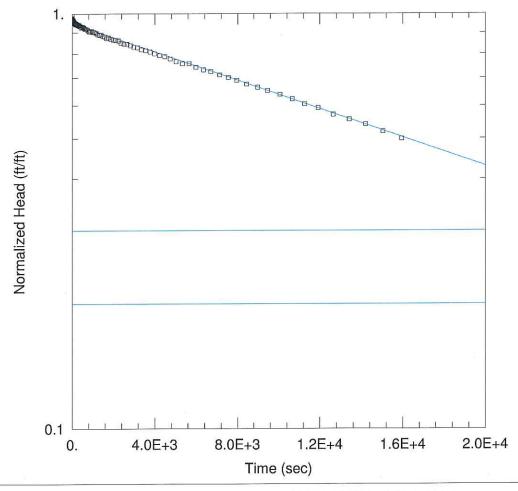
SOLUTION

Aquifer Model: Confined

K = 4.0E-6 cm/sec

Solution Method: Bouwer-Rice

y0 = 0.6908 ft



SLAG 2 DEEP RISING HEAD SLUG TEST (SLUG-OUT)

Data Set: \...\Slag 2 Deep Slug Out.aqt

Date: 12/15/16

SLUG 2B

Time: 08:45:07

PROJECT INFORMATION

Company: Barr Engineering Co. Client: OtterTail Power Company

Project: 41251005 Location: Big Stone, SD Test Well: Slag 2 Deep Test Date: Aug 12, 2016

AQUIFER DATA

Saturated Thickness: 7. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 2 Deep)

Initial Displacement: 1.202 ft

Total Well Penetration Depth: 7. ft

Casing Radius: 0.083 ft

Static Water Column Height: 19.26 ft

Screen Length: 5. ft Well Radius: 0.177 ft

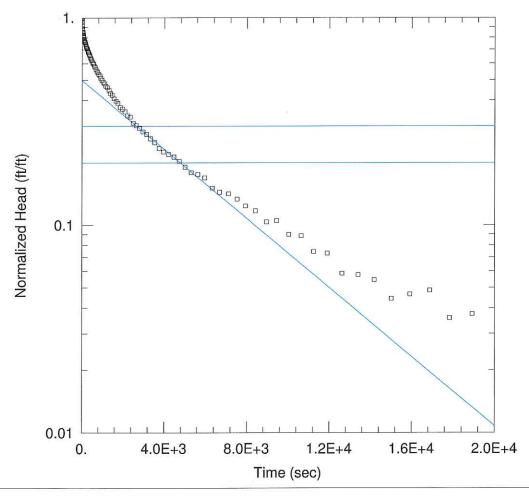
SOLUTION

Aquifer Model: Confined

K = 2.264E-6 cm/sec

Solution Method: Bouwer-Rice

y0 = 1.14 ft



SLAG-5 FALLING HEAD SLUG TEST (SLUG-IN)

Data Set: \...\Slag-5 Slug In.aqt

Date: 09/23/16

Time: 12:04:53

PROJECT INFORMATION

Company: <u>Barr Engineering Co.</u> Client: OtterTail Power Company

Project: 41251005

Location: Big Stone, SD

Test Well: Slag-5

Test Date: July 29, 2016

AQUIFER DATA

Saturated Thickness: 3. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 5)

Initial Displacement: 0.897 ft

Total Well Penetration Depth: 3. ft

Casing Radius: 0.083 ft

Static Water Column Height: 15.43 ft

Screen Length: 3. ft Well Radius: 0.344 ft

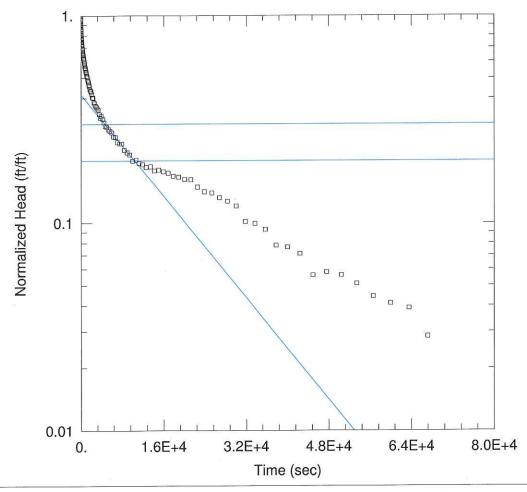
SOLUTION

Aquifer Model: Confined

K = 1.038E-5 cm/sec

Solution Method: Bouwer-Rice

y0 = 0.4478 ft



SLAG-5 RISING HEAD SLUG TEST (SLUG-OUT)

Data Set: \...\Slag-5 Slug Out.aqt

Date: 09/23/16 Time: 12:05:50

PROJECT INFORMATION

Company: <u>Barr Engineering Co.</u> Client: OtterTail Power Company

Project: 41251005 Location: Big Stone, SD Test Well: Slag-5 Test Date: July 30, 2016

AQUIFER DATA

Saturated Thickness: 3. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 5)

Initial Displacement: <u>0.949</u> ft Total Well Penetration Depth: <u>3.</u> ft

Casing Radius: 0.083 ft

Static Water Column Height: 15.43 ft

Screen Length: 3. ft Well Radius: 0.344 ft

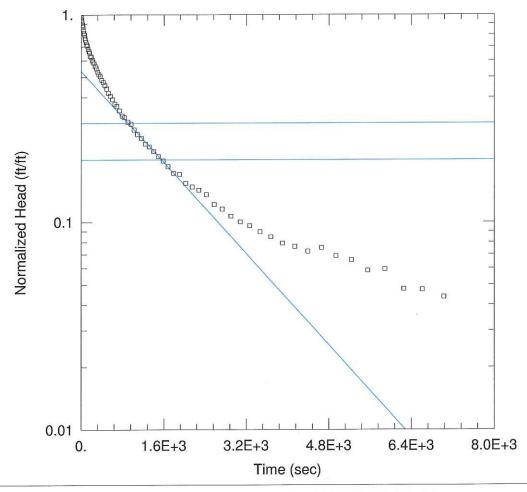
SOLUTION

Aquifer Model: Confined

K = 3.817E-6 cm/sec

Solution Method: Bouwer-Rice

y0 = 0.3951 ft



SLAG 8 FALLING HEAD SLUG TEST (SLUG-IN)

Data Set: \...\Slag 8 Slug In.aqt

Date: 09/23/16 Time: 12:07:58

PROJECT INFORMATION

Company: <u>Barr Engineering Co.</u> Client: OtterTail Power Company

Total Well Penetration Depth: 7. ft

Casing Radius: 0.083 ft

Project: 41251005 Location: Big Stone, SD Test Well: Bonus Well Test Date: July 27, 2016

AQUIFER DATA

Saturated Thickness: 7. ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 8)

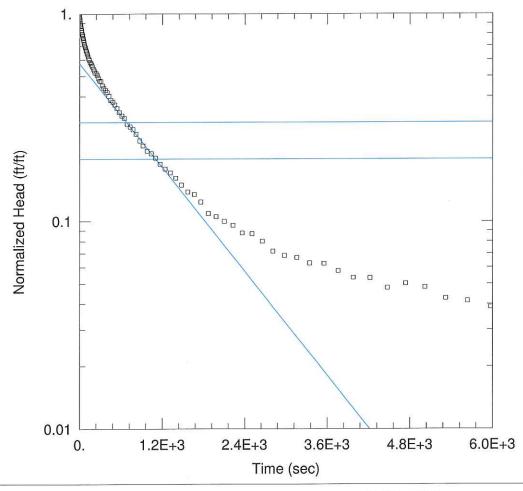
Initial Displacement: 1.016 ft Static Water Column Height: 35.31 ft

Screen Length: <u>5.</u> ft Well Radius: 0.177 ft

SOLUTION

Aguifer Model: Confined Solution Method: Bouwer-Rice

K = 3.621E-5 cm/sec y0 = 0.5422 ft



SLAG 8 RISING HEAD SLUG TEST (SLUG-OUT)

Data Set: \...\Slag 8 Slug Out.aqt

Date: 09/23/16 Time: 12:08:49

PROJECT INFORMATION

Company: Barr Engineering Co. Client: OtterTail Power Company

Project: 41251005 Location: Big Stone, SD Test Well: Bonus Well Test Date: July 27, 2016

AQUIFER DATA

Saturated Thickness: 7. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 8)

Initial Displacement: 1.123 ft

Total Well Penetration Depth: 7. ft

Casing Radius: 0.083 ft

Static Water Column Height: 35.31 ft

Screen Length: 5. ft Well Radius: 0.177 ft

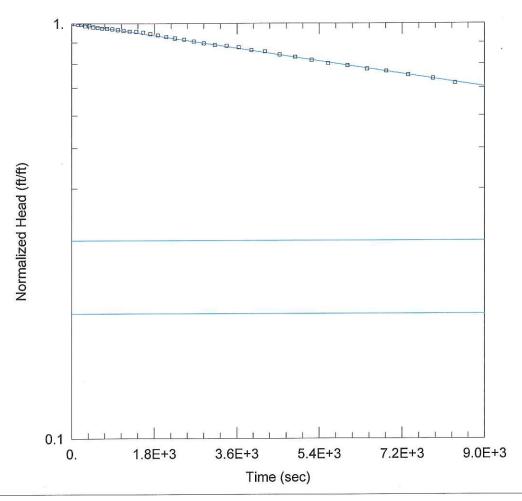
SOLUTION

Aquifer Model: Confined

K = 5.505E-5 cm/sec

Solution Method: Bouwer-Rice

y0 = 0.6481 ft



SLAG 9 DEEP RISING HEAD SLUG TEST (SLUG-OUT)

Data Set: \...\Slag 9 Deep Slug Out.aqt

Date: 10/26/16 Time: 16:22:44

PROJECT INFORMATION

Company: <u>Barr Engineering Co.</u> Client: OtterTail Power Company

Project: 41251005

Location: Big Stone, SD Test Well: Slag 9 Deep Test Date: Aug 12, 2016

AQUIFER DATA

Saturated Thickness: 7. ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Slag 9 Deep)

Initial Displacement: 1.466 ft

Total Well Penetration Depth: 7. ft

Casing Radius: 0.083 ft

Static Water Column Height: 15.61 ft

Screen Length: <u>5.</u> ft Well Radius: 0.177 ft

SOLUTION

Aquifer Model: Confined

K = 2.247E-6 cm/sec

Solution Method: Bouwer-Rice

y0 = 1.47 ft